

# Producer statement design (PS1)



**TO BE COMPLETED BY THE DESIGN PROFESSIONAL WHO HAS BEEN ENGAGED TO PROVIDE A PS1**

Author name: **Lyall Green** Author number: **1274**

Author company: **Design Management Consultants Ltd**

Building consent N:

Site address: **52 Kauri Point Road Laingholm Waitakere**

Legal description: **Lot 402, DP 17523 CT NA899/143**

Engaged by: **B & C Morton-Jones** (Owner)

To provide design services in respect of (describe work):  
 part **structural design of house extension foundation, floor framing and roof beams, subfloor bracing and deck extension balustrade**  
 all

NZBC clauses (circle as applicable)

NB: all statements must include B2

<b>(B1)</b>	B2	C1	C2	C3	C4	C5	C6	D1	D2	E1	E2	E3
F1	F2	F3	F4	F5	F6	F7	F8	G1	G2	G3	G4	G5
G6	G7	G8	G9	G10	G11	G12	G13	G14	G15	H1		

The design has been prepared in accordance with:

Documents issued by the Ministry of Business, Innovation & Employment

**B1 / VM1**  
(verification method / acceptable solution)

Alternative solution (attach schedule if required)

The proposed building work covered by this producer statement design is described on the drawings referenced below together with the specifications and other documents set out in the schedule attached to this statement:

Drawing title: **Proposed Alterations to 52 Kauri Point Road, Laingholm** Drawing numbers: **A106 revA, A107 revA, A108 revA, A323revA**

The producer statement is subject to:

(i) Site verification of the following design assumptions: **ULS soil bearing capacity of 230kPa**

(ii) All proprietary products meeting their performance specification requirements.

I believe on reasonable grounds that the proposed building work will comply with the relevant provisions of the Building Code if constructed in accordance with the drawings, specifications and other documents provided or listed with this statement.

I understand that Auckland Council is reliant on this producer statement for the purposes of establishing compliance with the relevant provisions of the Building Act 2004, Building Regulations and Building Code. I confirm that I hold a current policy of professional indemnity insurance to the value required by Auckland Council.

Construction monitoring is:

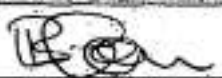
Not required

Required (if required please list below)

Verification of soil bearing strength and foundation construction

(Agreement must be attached)

Signed by:



Date:

14/04/2015

AGENZ  IPENZ  PENZ  NZIAS  Other (specify)

46696

(Membership number)

Address:

PO Box 5254 Hamilton

Postcode: 3242

Phone:

07 850 5155

Fax:

Mobile:

Email:

lyall.green@dmcnz.com

#### COMMENTS

All construction to be in accordance with NZS 3604 and the New Zealand Building Code

#### Important notes:

- Producer statements are accepted solely at Auckland Council's discretion; please refer to the Producer Statement Policy which can be found on Council's website for further details  
<http://www.aucklandcouncil.govt.nz/EN/rates/building/property/consents/Consent%20documents/ac2301producerstatementpolicy.pdf>

Producer statement design  
(PS1)

**SUPERSEDED**

**Auckland Council**  
Te Kaitiaki Take Kōwhiri



TO BE COMPLETED BY THE DESIGN PROFESSIONAL WHO HAS BEEN ENGAGED TO PROVIDE A PS1

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 all

NZBC Clauses (circle as applicable)

(B1)	B2	C1	C2	C3	C4	C5	C6	D1	D2	E1	E2	E3
F1	F2	F3	F4	E5	F6	F7	F8	G1	G2	G3	G4	G6
G6	G7	G8	G9	G10	G11	G12	G13	G14	G15	H1		

NB all statements must include B2

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Documents issued by the Ministry of Business, Innovation & Employment: **B1 / VM1** (verification method / acceptable solution)

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Date:

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ACENZ  IPENZ  CPENG  NZIA  Other (specify)

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Job No: 0008  
Project: B & C Morton-Jones House extensions  
Deck Baluster - refer sheet A308

PAGE: 1  
DATE: 21-Jun-15  
BY: LG

**Loads**

	Top edge and rail			Infill	
	Inwards Horizontal kN/m	Vertical kN/m	Inwards, outwards kN	Horizontal kPa	Any direction
Other residential (see also C)	0.75	0.75	0.60	1.00	0.50

**Posts**

Post size = ex 100\*100 94 mm  
Spacing = 1.00 m max  
Rail height above centre of supp = 1.20 m max  
U = 1.5Q > M\* = 0.90 kN-m  
94\*94 post > Zx = bd<sup>2</sup>/6 = 138431 mm<sup>3</sup>  
F\* = 6.50 Mpa  
Timber grade = H5 G8  
I wet condition > Fb = 11.70 Mpa  
phi = 0.80  
ki = 0.80  
phi Fb = 7.49 Mpa > F\* Ok  
Use ex 100\*100 G8 post at 1.0m max c/c



**Top rail**

Rail = 100\*50\*4.5 alloy channel UA 1146 TS min yield = 110.00 Mpa  
Ix = 1309377 mm<sup>4</sup>  
Zx = 26188 mm<sup>3</sup>  
Loading = 0.75 kN/m  
Span = 1.00 m max  
U = 1.5Q > M\* = 0.14 kN-m  
F\* = 5.37 Mpa  
TS alloy phi = 0.90  
ki = 1.00  
phi Fb = 99.00 Mpa > F\* Ok  
Use 100\*50\*4.5 TS alloy rail for post centres at 1.0m max

**In-fill rails**

Rails = 20mm dia tube - UA 1325 6060 TS min yield = 110 Mpa  
r = 1.20 mm  
Ix = 3143 mm<sup>4</sup>  
Zx = 157 mm<sup>3</sup>  
spacing = 110.00 mm max c/c  
span = 1.05 m  
Loading = 0.11 kN/m  
U = 1.5Q > M\* = 0.02 kN-m  
F\* = 96 Mpa  
6060 TS alloy phi = 0.90  
ki = 1.00  
phi Fb = 99.00 Mpa > F\* Ok  
Use 20mm dia\*1.2mm TS 6060 alloy tube infill rails @ 110mm max centres

**Fasteners to infill rails**

Face loading = 1.00 kPa  
U = 1.5Q > V\* = 0.06 kN = potential screw pull out load  
Screws = 75mm \*4.88 (10g)  
Withdrawal strength = 61.70 N/mm  
Embedment = 45.00 mm  
k (green/wet timber) = 0.8\*0.8\*1 = 0.64  
phi = 0.70  
phi Qs = 1243.87 N / screw Ok  
Use 75mm \* 4.88mm stainless steel screws at base of tubes

**Post support**

100\*100 post @ 1m max c/c with applied load 1.0+0.1 = 1.1mm above top fastener bolt  
Boundary joist are ex 200\*50mm \*2  
Distance from applied load to base of post = 1.0+0.4\*1.9 = 1.23 m  
Max moment from applied load to post base = 0.92 kN-m  
Distance from base to top bolt = 0.13 m  
Max load on bolts = 7.10 kN  
Use 2 M12 SS bolts complete with 50mm washers as down



Design Management Consultants  
Limited

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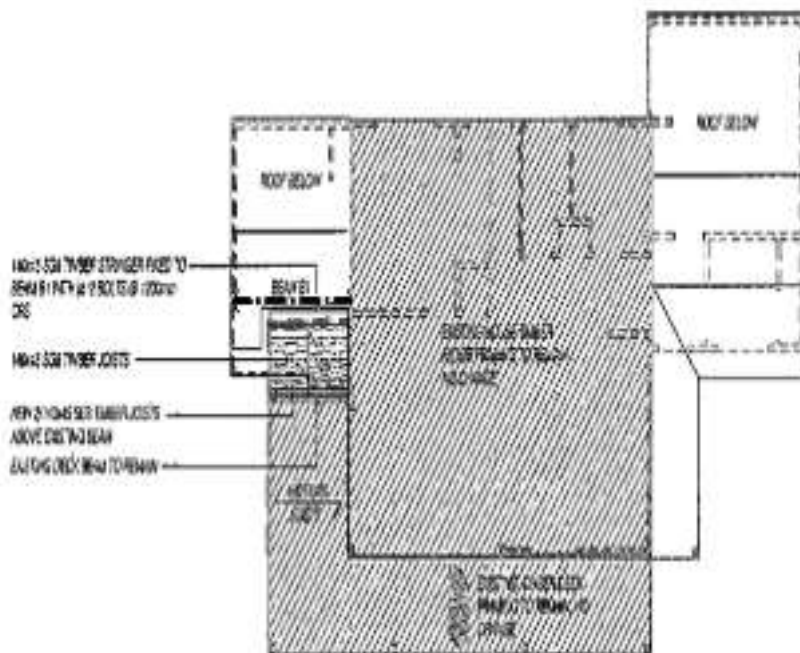
# Structural Calculations

**Project: B & C Morton-Jones  
House Extensions**

**Project Ref: 9006**

the fusion of engineering and architecture 





**FLOOR FRAMING NOTES**

- ALL WORK TO COMPLY WITH RELEVANT CLAUSES OF THE ABC AND THE NEW BUILDING ACT
- ALL FLOOR AND SUBFLOOR FRAMING TO COMPLY WITH NZS 3604:2011 SECTION 4.1.4. ALL WORK OUTSIDE THE SCOPE TO ENGINEER'S DISCRETION
- READ DRAWINGS IN CONJUNCTION WITH DESIGNER'S NOTES TO SPECIFICATION
- READ DRAWINGS IN CONJUNCTION WITH ENGINEER'S CALCULATIONS
- ALL WORKMANSHIP TO BE CARRIED OUT TO BEST PRACTICE WITH ALL MATERIALS USED TO THE BEST OF THEIR RESPECTIVE TRADES

**TIMBER TREATMENT**

ALL SEVERED FLOOR AND SUBFLOOR FRAMING TO BE OF THE FOLLOWING:

- ALL FLOOR JOISTS, BEAMS, LAMINATES AND ORIGINAL BRACES AND ASSOCIATED FRAMING & INTERMEDIATE FLOOR FRAMING TO BE TREATED WITH ALL EXTERNALLY EXPOSED POSTS & FRAMING ABOVE GROUND TO BE TREATED TO G2
- ALL PILE, POSTS AND POLES BASED ON IN THE GROUND AND FLOOR JOIST FRAMING TO BE TREATED TO G2
- ALL TIMBER JOIST FRAMING TO COMPLY WITH G2

**TIMBER GRADING**

ALL STRUCTURAL TIMBER IS TO BE STRESS GRADED TO COMPLY WITH THE CURRENT EDITION NZS 3604 AND MARKED ACCORDINGLY TO COMPLY WITH THE CURRENT EDITION NZS 3604. ALL TIMBER SIZE ARE POINTED FROM THE 25.4 TABLE FROM NZS 3604:2011. BELOW TABLE THEREAFTER ALL STRUCTURAL TIMBER SHOULD BE G2 UNLESS OTHERWISE SPECIFIED

**STRUCTURAL FIXING LEGEND**

ALL STRUCTURAL FRAMES SHALL BE TIED & EXPLICIT DIMENSIONS GIVEN. SUBFLOOR AS SHOWN IN THIS DRAWING ARE TO BE BRIDGED AT ALL STRUCTURAL FRAMES IN ALL CLOSED ENVIRONMENTS TO BE NOT OPEN.

**FLOOR FRAMING LEGEND**

--- NEW 150mm TIMBER JOISTS, SIZE AS SPECIFIED

--- NEW 150mm TIMBER BEAMS, SIZE AS SPECIFIED

○ POINTED FROM ABOVE

**FLOOR FRAMING LEGEND**

EXISTING HOUSE FLOOR JOIST FRAMING TO REMAIN

NEW JOIST FRAMING WITH NEW JOISTS TO MATCH EXISTING

*Handwritten:*  
 H.H. 15  
 J.R.  
 CE Eng #46696

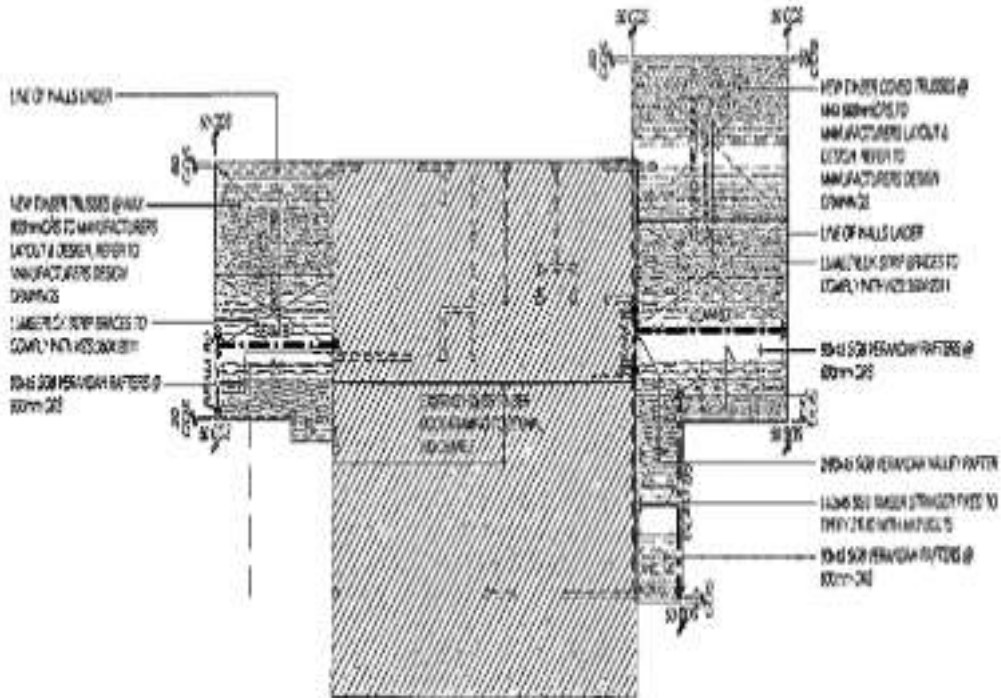
	25 250 0000 1000 00 00 00 0000 00 00 00 00 0000 00 00 00 00 00 00 00 00 00 00 00 00 00 00 00 00 00 00 00		THE ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF ANY WORK WHICH IS NOT SPECIFICALLY REFERRED TO IN THIS CONTRACT. THE ENGINEER'S RESPONSIBILITY IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE WORK REFERRED TO IN THIS CONTRACT.	CONTRACT NO. PROJECT NO. DRAWING NO. DATE	AS PER PROJECT CONDITIONS TO STANDARD CONTRACT WORKS FOR HOUSES HORTON-JONES	SHEET NO. 1-01 FLOOR FRAMING PLAN	QUANTITY SCALE WEIGHT 14-000 1:100 1.00kg
	14-000 1:100 1.00kg		14-000 1:100 1.00kg	14-000 1:100 1.00kg			



NOTE ALL ROOF DRAWINGS TO MATCH EXISTING CHECK DATE

7x4 TIMBER TRUSSES ON PACE @ 800mm C/S

7x4 TIMBER CEILING BATTENS @ 400mm C/S



**ROOF FRAMING NOTES**

- ALL WORKS TO COMPLY WITH THE FIRST CLASS OF THE NCC AND THE NEW ZEALAND BUILDING ACT.
- TIMBER FRAMED TRUSSES TO MANUFACTURERS DESIGN LAYOUT, UNLESS OTHERWISE SPECIFIED FOR TRUSS MANUFACTURERS INFORMATION TRUSS MANUFACTURER TO PROVIDE PROUDER STATEMENT
- ALL ROOF FRAMING & BRACING TO COMPLY WITH NZS BRACING SECTION 5.6 TO ALL FRAMING & BRACING OUTSIDE THIS SCOPE TO ENGINEERS DESIGN
- READ DRAWINGS IN CONJUNCTION WITH DESIGNER'S SPECIFICATION
- READ DRAWINGS IN CONJUNCTION WITH ENGINEER'S CALC & DRAWING
- ALL WORKMANSHIP TO BE CHECKED OUT TO BEST PRACTICE PRACTICE WITH ALL WORKMANSHIP TO BE THE BEST OF THEIR RESPECTIVE CRAFTS

**TIMBER TREATMENT:**

ALL ROOF FRAMING TO BE OF THE FOLLOWING ALL ROOF FRAMING TO BE M3 TREATED TIMBER UNLESS OTHERWISE SPECIFIED OR GRANTED ALL TIMBER CAPABILITY TO COMPLY WITH USING

**TIMBER GRADING:**

ALL STRUCTURAL TIMBER USED TO BE STRESS GRADED TO COMPLY WITH THE CURRENT SECTION USE 230 AND MARKED ACCORDINGLY TO COMPLY WITH THE CURRENT SECTION AND ALL TIMBER USES ARE WORKED FROM THE BEST TRUSSES FROM ACROSS THE ENTIRE LENGTH THEREFORE ALL STRUCTURAL TIMBER SHOULD BE SOLID UNLESS OTHERWISE SPECIFIED

**BEAM/LINE LEGEND**

81

**ROOF FRAMING LEGEND**

EXISTING HOUSE ROOF FRAMING TO REMAIN

NEW ROOF FRAMING

**STRUCTURAL FRAMING LEGEND**

ALL STRUCTURAL FRAMING SHALL BE NEW TREATED TIMBER UNLESS OTHERWISE SPECIFIED

ALL STRUCTURAL FRAMING SHALL COMPLY WITH NZS BRACING SECTION 5.6 TO ALL FRAMING & BRACING OUTSIDE THIS SCOPE TO ENGINEERS DESIGN

**ROOF FRAMING LEGEND:**

NEW ROOF FRAMING TO TRUSS MANUFACTURERS DESIGN

NEW TIMBER TRUSSES TO TRUSS MANUFACTURERS DESIGN

NEW TIMBER TRUSSES TO TRUSS MANUFACTURERS DESIGN, SEE AS SPECIFIED

8 POST LINE FROM JOINT

**BEAM/LINE LEGEND**

2x4x8 SSI 7x4x8 TIMBER BEAM

2x4x8 SSI 7x4x8 TIMBER BEAM

2x4x8 SSI 7x4x8 TIMBER BEAM

2x4x8 SSI 7x4x8 TIMBER BEAM

2x4x8 SSI 7x4x8 TIMBER BEAM

*Handwritten notes:*  
4/11/15  
Ben  
Clary # 46696

	<p>IN ACCORD WITH THE BUILDING ACT 2004 AND THE BUILDING REGULATIONS 2004, THIS PLAN HAS BEEN PREPARED BY A REGISTERED PROFESSIONAL ENGINEER.</p> <p>REGISTERED PROFESSIONAL ENGINEER</p> <p>REGISTERED PROFESSIONAL ENGINEER</p> <p>REGISTERED PROFESSIONAL ENGINEER</p>	<p>STRUCTURAL ENGINEER</p> <p>REGISTERED PROFESSIONAL ENGINEER</p> <p>REGISTERED PROFESSIONAL ENGINEER</p>	<p>DATE: 14/03/15</p> <p>SCALE: AS SHOWN</p>	<p>PROJECT NAME: ROOF FRAMING PLAN</p> <p>CLIENT: [REDACTED]</p> <p>ADDRESS: [REDACTED]</p>	<p>DRAWN BY: [REDACTED]</p> <p>CHECKED BY: [REDACTED]</p>
					<p>DATE: 14/03/15</p> <p>SCALE: AS SHOWN</p>

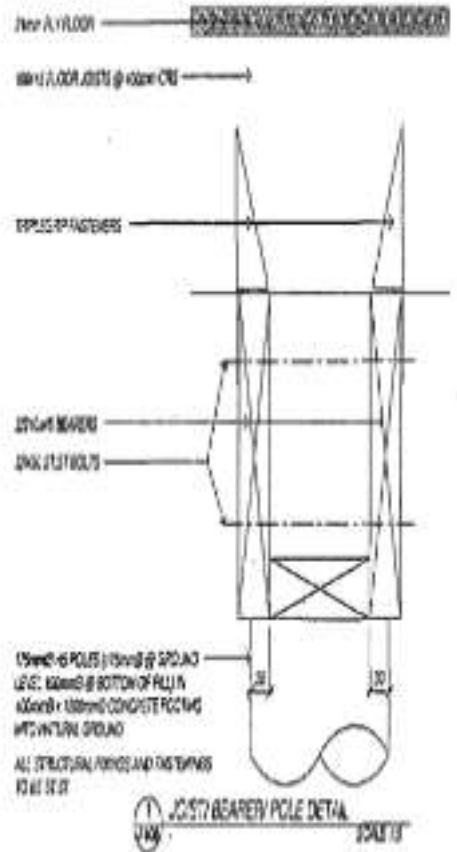
Table 2.2 - Filing type and capacity reference guide (see 2.3.3.3)

Filing type	Description	Alternative filing capacity (MM)	See table
A	2/60 x 3/6 end nails	0.7	
B	2/60 x 3/6 end nails + 2 wire clips	4.7	2.18
C	2/60 x 3/6 end nails + strap filing (see figure 2.12)	6.5	
D	4/60 x 2/6 end nails + 2 strap filing (straps tied)	10.8	
E	2/60 x 3/6 steel nails + 2 wire clips	4.7	10.1 413.1 10.7 413.2 10.11 413.11 10.12
F	2/60 x 3/6 steel nails + strap filing (see figure 2.12)	7.0	15.8 415.6 15.10 415.10
G	10/60 x 3/6 nails (6 each side)	4.7	
H	1/4" (12) ac h	8.5	10.7 413.0 10.7 415.7
I	2/4" (12) ac h	9.8	
J	2/4" (12) ac h	14.0	
K	6/16 x 3/8 nails	3.0	
L	2/4" (12) ac h	3.0	10.1 413.0
M	2/4" (12) ac h	13.0	
N	6/16 x 3/8 - 10G nails (hard silver)	4.7	
O	5/16" (8) ac h (see figure 8.3 (C))	6.8	10.8 415.3 10.8 415.3
P	2/4" (12) ac h (see figure 8.3 (B))	13.7	
Q	2/4" (12) ac h (see figure 8.3 (A))	25.5	
R	1/4" (12) ac h	3.0	
S	2/4" (12) ac h	2.8	10.10 413.10 10.12
T	1/4" (12) ac h - drilling screws, 80 mm long	2.1	10.8 415.6
U	1/4" (12) ac h - drilling type 17 screws, 100 mm long	5.5	

NOTE - Capacities are associated with filing type, not to screws. See individual section tables for the appropriate filing type for the application.

FRAMING RING REQUIREMENTS

- RING FOR TOP PLATES TO SUPPORTING MEMBERS (SIC) AT STAIR AND WELLS
- RING TYPE T (TABLE 2.1) (25/60)
- RING FOR RAILS
- RING TYPE T (TABLE 2.1) (25/60)
- RING FOR GUTTERS
- RING TYPE T (TABLE 2.1) (25/60)



14/4/15  
BR  
City # 46696



Design Management Consultants Ltd  
PO Box 5254, Frankton, Hamilton 3242  
Phone: 07 850 5155 Fax: 07 850 5158

Project Ref: 4006 - 2015  
Project: Kauri Pt Rd Whitakere  
Client: B & C Merton Jones  
Date: 12/15 Page: 1

Author CA

House Extensions  
Add - Bedroom @ east side  
- Bedroom & laundry @ west side

Refer to dmc geotech report 30/15  
→  $q_{ave} = 230 \text{ kPa}$

Refer to the Drafting Machine's drawings 14-029

Design Loads

① Dead New light weight roof =  $0.35 \text{ kPa}$   
Previous calc have existing roof @  $0.35 \text{ kPa}$   
- 4 decks @  $0.3 \text{ kPa}$

② Live  $1.5 \text{ kPa}$

③ Wind From previous calc - wind calc 15/11  
Max directional wind speed =  $42.1 \text{ m/s}$

④ Seismic Cab bracing & cantilever poles  
Ductility =  $1.25$   
Class C walls  $\mu = 0.14$   
 $Z = 0.13$   
 $C_d(F) = 2.71$   
 $S_p = 1.3 - \frac{0.2 \times 1.25}{0.7} = 0.925$   
 $F_{w1} = \frac{1.25 \times 1.25}{0.7} + 1 = 1.014$   
 $C_d = \frac{0.13 \times 2.71 \times 0.925}{1.014} = 0.25$  \*

$V = C_d W_T$      $W_T = C + U_e Q$   
 $Q = U_e A + 1.5$      $U_e = 0.3 + \frac{3}{A}$

Existing floor area = 2 levels  $52 \text{ m}^2 = 104$   
New area =  $38$   
Total area =  $142 \text{ m}^2$

Total House with extensions

$U_e = 0.55$   
 $W_T = C + (0.3 \times 0.55) Q$   
 $C = 142 \times (0.55) + 2 \text{ levels walls @ } 0.3 \text{ kPa}$   
Area walls =  $3 + 5 \times 4 + 0.2 + 8.5 \times (3.8 + 5) + 2.8 + 3 \times 1$   
 $= 270 \text{ m}^2$   
 $C = 142 + 0.35 + 270 \times 0.3 = 131 \text{ kN}$

$W_T = 131 + 0.55 \times 1.5 \times 0.3 + 142 = 166 \text{ kN}$   
Plus decks    say  $180 \text{ kN}$



Total seismic lateral load

$$\begin{aligned}
 U &= F_w \\
 &= C_d W_A \\
 &= 0.25 \times 180 = 45.0 \text{ kN} \\
 &= 900 \text{ BCUs}
 \end{aligned}$$

Author

Wind lateral load - Sheltered South & West.

$$V_d = 42.1 \text{ m/s}$$

$$q_d = 1106 \text{ N/m}^2$$

$$\frac{q_d}{\rho} = > 0.3$$

$$C_{p,nett} = +0.7 + 0.5 = 1.2$$

$$\begin{aligned}
 \text{North-East face area} &= (3.5 \times 4) + 2.9 \\
 &\quad + 0.5 \times 5.7 \\
 &= 72.0 \text{ m}^2
 \end{aligned}$$

$$\begin{aligned}
 U &= W_{trans} \\
 &= 1106 \times 1.2 \times 72 = 946 \text{ kN} \\
 &= \text{transverse wind load} = 1932 \text{ BCUs}
 \end{aligned}$$

$$\begin{aligned}
 \text{South East face area} &= 6.2 \times 5.4 + 1.5 \times 2.9 \\
 &= 37 \text{ m}^2
 \end{aligned}$$

$$\begin{aligned}
 U &= W_{long} \\
 &= 1106 \times 1.2 \times 37 = 47.1 \text{ kN} \\
 &= 942 \text{ BCUs}
 \end{aligned}$$

Wind load governs lateral bracing design in both transverse & longitudinal directions.



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Client: \_\_\_\_\_

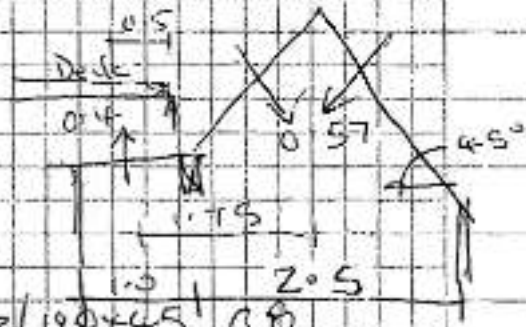
Date: 1/1 Page: 3

Author

Roof Beams B1 - B4  
 $C = 0.35 kPa$  Roof & Deck  
 $D = 0.25 kPa$   
 $W_u = q = 1.06 kPa$

(B1) Span = 3.0m  
 Effective width = 2.0m Roof plus 0.5m deck  
 Roof pitch = 30°  
 $C_{pe} = 0.8$  sun  $\alpha = +0.57$   $\alpha = 45^\circ$   
 $C_{pe} = -0.9$  or  $-0.4$  for  $\alpha = 30^\circ$

Net result on B1 is  
 $1.06 (-0.4 + 0.8 + 0.57 \times 1.25)$   
 $= +0.55 kN/m$   
 $U = C + W_u$   
 $= (1.15 + 0.5) \times 3 + 0.55$   
 $= 1.4 kN/m$   
 Refer Spreadsheet - Use 2/140x45 CB



(B2) Supports end of B1 over window



Refer Spreadsheet - Use 2/140x45 CB.



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Project Ref: \_\_\_\_\_

Project: \_\_\_\_\_

Client: \_\_\_\_\_

Date: \_\_\_\_\_ Page: 4

Author

Roof beam (B3)

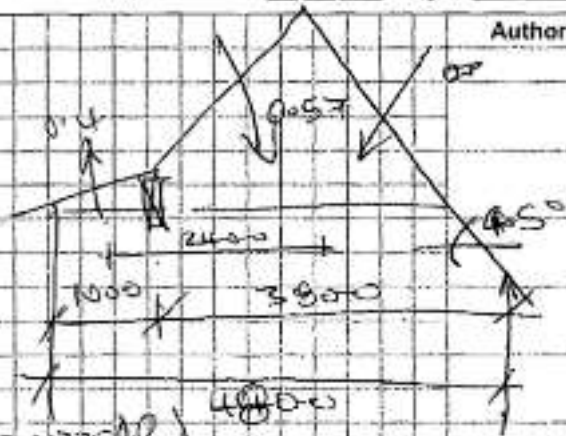
Span = 4400  
 EOP width = 2400

Wind Cpe.

Nett wind load on B3 is

$$1.06(-0.4 \times 0.5 + 0.57 \times 1.9) \\ = +0.9 \text{ kN/m (for } 2400 = 0.375 \text{ kPa)}$$

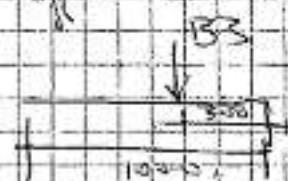
$$U = G + W_u = 2.0 \times 0.4 + 0.9 \\ = 1.85 \text{ kN/m}$$



Refer spreadsheet rules - Use 2/240x45 GB

Roof beam (B4) - Supports B3.

Span = 1000  
 EOP width = 0



Refer spreadsheet rules - Use 2/140x45 GB

Floor Beams

Beam (2)

Floor Joists - Span = 2670  
 refer N28 360x45 → 140x45 @ 400 c/c

- Double up under walls and in line with window trim B1 load

Beam A - Refer spreadsheet

Use 2/240x45  
 span = 700 - 3000 - 700



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Project Ref: \_\_\_\_\_

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Client: \_\_\_\_\_

Date: 1/1 Page: 5

Floor Beams contd.  
 Bed 3

Author

Floor joists  $S_{span} = 2.2m$   
 Use 140x65 @ 450 c/c.

Beam B (Central beam)  
 $width = 2.2m$   
 $S_{span} = 3.0m$   
 Refer to Microstran for results  
 $U = 1.2G + 1.5Q \Rightarrow M^* = 50.8 \text{ kNm}$   
 Use 2/200x45 CB

Beam C (Outer beams)  
 $width = 1.1m$   
 $S_{span} = 3.0m$   
 Refer to Microstran for results  
 $U = 1.2G + 1.5Q \Rightarrow M^* = 4.0 \text{ kNm}$   
 Use 2/200x45 CB.

Perimeter Floor joists (double span)  
 $M^*_{max} = 1.96 \text{ kNm}$   
 2/140x65  $\Rightarrow \phi_{min} = 2.0132$   
 $= 2.64$   
 $> 1.96 \text{ OK}$

Use double joists under exterior walls.

Roof Beam (S) - Supports facing verges exposed to wind.

$S_{span} = 3.0$   
 effective width = 0.6  
 $\mu_{lic} = 0.4$   
 $C_{pe} = +0.5 \text{ @ } +0.2$   
 $q = 1.06 \text{ kPa}$   
 $W_{x1} = +0.53 \text{ kPa}$   
 $W_{x2} = -0.21 \text{ kPa}$

Refer spreadsheet  
 Use 140x65 CB.



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Date: 1/1 Page: 6

Author

Lateral Loads

Existing house is fully braced for its own lateral loads

Provide lateral bracing for each portion of extensions (at each end of house)

Red 2. - South east exposure NB  

$$\text{Wind Load} = 0.7 \times (1 - 0.6) \text{ kPa} \times 0.9^2$$

$$= 0.60 \text{ kPa}$$

Max exposed face =  $3.0 \times 4.0 = 12 \text{ m}^2$

$$W_w = 7.22 \text{ kN}$$

Sway load  $W_s = C_s W_w Q$   
 Provision rules  $C_s = 0.25$  ( $\mu = 1.25$ )  

$$W_s = 0.4 (2 \times 2 + 3.0 \times (4 + 2 \times 0.5)) + 12 \text{ m}^2 \times 0.3 \times 1.5 \text{ kPa}$$

$$V = C_d W_s = 28.2 \text{ kN}$$
  

$$V = C_d W_s = 7.1 \text{ kN}$$

Wind governs lateral bracing design.

$$W_w = 7.22 \text{ kN}$$

OR  $3.61 \text{ kN/pole}$

Utilise post-tension timber poles  
 Height ground to floor = 2500

Eurotech report indicates fill depth 1 to 1.4m below ground.

Axial load on poles  $V = 1.2 C_s W_s Q = 21.6 \text{ kN}$

$$Q_{\text{req}} = 21.6 \text{ kPa}$$

$$A \geq \frac{21.6}{1.25} = 0.094 \text{ m}^2$$

$$\Rightarrow 346 \text{ mm}$$

Use 400  $\phi$  concrete encasement.

Refer Buchanan's embedment analysis for

Clays  $C_u = 70 \text{ kPa}$

$$e = 2.5 \times f_{cl} = 3.9 \text{ m}$$

Embedment below fill =  $1.2 \text{ m} \times 400 \phi$





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Pole design

Bed 2,  $H_{i \max} = 3.61 \text{ kN/pole (wind)}$

$$e = 2.5 \times 1.4 = 3.9 \text{ m}$$

$$M = 3.61 \times 3.9 = 14.1 \text{ kNm max.}$$

For Normal pole ( $350 \text{ kg/m}^3$ )

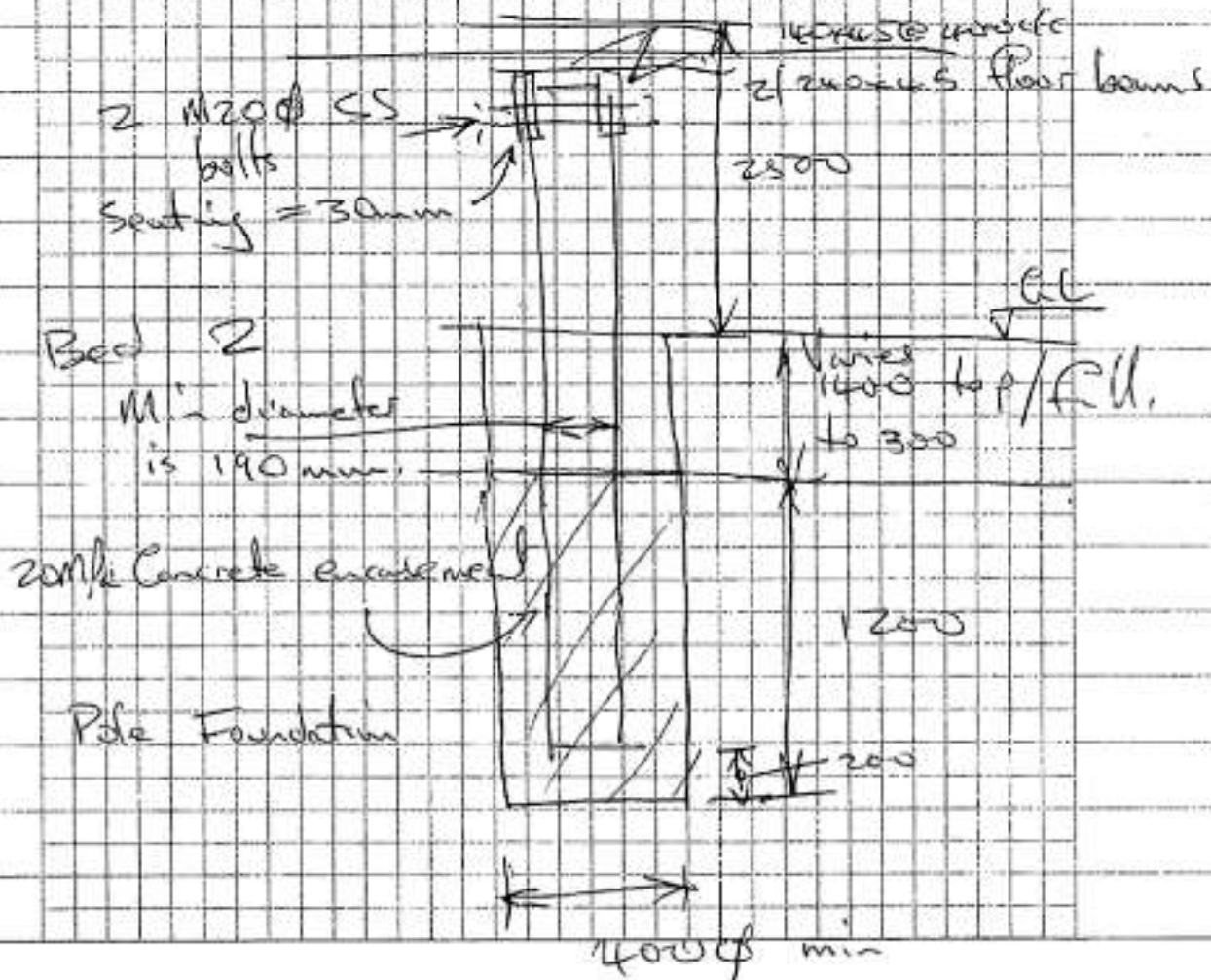
$$W = 0.8 \text{ m} \times 1.0 \times (38 \times 0.85^2)$$

$$= 2.2 \text{ kPa}$$

$$B = \frac{M}{W} = \frac{14.1}{2.2} = 6.4 \text{ cm}^2$$

$$= \frac{\pi d^2}{4} \geq 180.7 \text{ cm}^2$$

Use 200 @ SED poles  
 or 175 @ with 190mm below ground selected.





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Bas 3

Wind Sheltered from all wind except open to North east wind

$$\text{Net wind load} = 0.17 \times (1.06 \times 0.9^2) = 0.16 \text{ kPa}$$

$$\text{Max exposed face walls} = 4.92 \times 3.1 \text{ m}^2 = 15.3 \text{ m}^2$$

$$\therefore \text{Lateral Wind} = 15.3 \times 0.16 = 2.45 \text{ kN}$$

Seismic

Refer previous rules  $C_s = 0.125$  ( $\mu = 1.25$ )

$$W_H = C_s \times C_d \times D \quad U_e = 0.3 \quad C_d = 1.5 \times C_s$$

$$C_d = 0.4 \times (3 \times 15.3 + 3.0 \times (4.9 + 3 \times 4.9)) = 3.4$$

$$W_H = 3.4 + 0.125 \times 1.5 \times 15.3 = 4.1 \text{ kN}$$

$$V = C_d W_H = 10.2 \text{ kN} \quad \text{Wind governs}$$

Lateral load is supported by 6 poles at various heights above sloping ground height varied 0.5m to 3.1m.

Refer Mycrastan model for analysis of pole distribution with Moment Release floor diaphragm.

From Mycrastan critical pole is 1.2m high

$$V_{max} = 3.62 \text{ kN}$$

$M_{max} = 4.1 \text{ kNm}$  refer to Bratons spreadsheet for embedment

1.2 Pole

$$e = 1.2 + 0.3 \text{ topsoil} = 1.5 \text{ m}$$

$$V = 3.62 \text{ kN} \quad C_u = 70 \text{ kPa}$$

$$\phi = 0.16 \text{ m}$$

$$L = \text{Embedment} \geq \frac{940 \text{ mm} \times 400 \phi}{\phi + 0.75 \phi} = 1200 \text{ mm}$$

0.5 Pole

$$e = 0.7 \quad L \geq \frac{990 \text{ mm} \times 400 \phi}{\phi} = 1200 \text{ mm}$$

3.1 Pole

$$e = 3.3 \quad L \geq 710 \times 400 \phi$$

$$V = 0.2$$

Use 1200mm x 400 phi embedment.



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Panel 3 poles =  
 $\frac{M}{W} = \frac{M}{W}$

$$M_{max} = 3.4 \times 0.5 = 5.1 \text{ kNm}$$
$$\Rightarrow \geq 232 \text{ cm}^3$$

$$d \geq 130.3 \text{ cm}$$

Use 175 SED poles.

\* 1200 mm x 400  $\phi$   
embedment



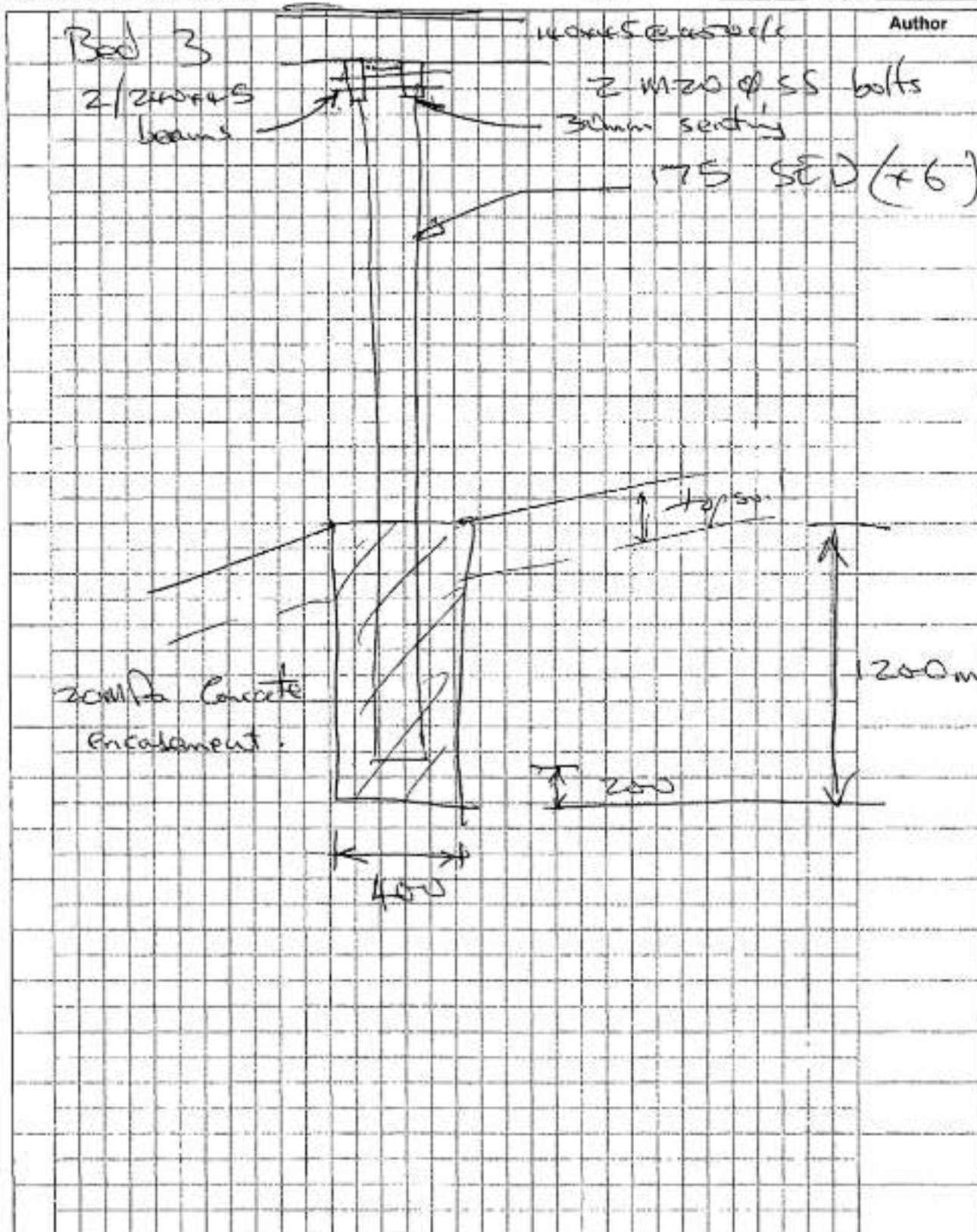
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## Design Management Consultants

JOB NAME: B & C Morton-Jones House  
 Job File Number: 9000  
 Description: Wind speed calcs

Page No.: 1  
 Date: 12/6/11  
 Designer: Lyall Green  
 Checker: 1

### Determination of Site Wind Speeds

Enter Return Period of Interest: 500

Determination of Site Wind Speed  $V_{s,r,p} = V_R * M_d * M_{z,r,p} * M_s * M_t$  m/sec

Enter Wind Region ID = A7

Wind Regime with this region is Frontal

Regional Wind Speed =  $V_R = 45$  Adjustment Factor = 1  $V = 45$  m/sec  $V_{1.2}$

Directional (d) or Non-directional (n) wind speeds

Elements being considered C. (C = Complete building & Primary Structure; C= Other elements)

Wind Directional Multiplier,  $M_d$ , for Region A7

N	NE	E	SE	S	SW	W	NW
0.9	0.9	0.8	0.9	0.9	0.9	1	1

#### Terrain/Height Multiplier

Enter Building Height  $Z = 9$  m  
 Enter Reference height of interest  $Z = 9$  m (can select several heights up the Building)  
 Terrain Category Averaging Distance  $1000$  m (As given in Table 4.2A)  
 Enter the distance of each  $1000$  m of the site

Terrain Category present over TC	N	NE	E	SE	S	SW	W	NW
Averaging Distance 1	2/6000	2/6000	2/6000	2/6000	2/6000	2/6000	3/6000	3/6000
Starting at base of building and								
Moving away in an upwind direction								
4								
(Refer to Clause 4.2.3)								
5								
6								
7								
8								
Sum of averaging distances	1187	1187	1187	1187	1187	1187	1105	1105
check distance provided > Required	OK	OK	OK	OK	OK	OK	OK	OK
Weighted average terrain category	2.00	2.00	2.00	2.00	2.00	2.00	3.00	3.00
$M(z, cat) =$	0.98	0.98	0.98	0.98	0.98	0.98	0.83	0.83

#### Shielding Effects

(Note Shielding Zone is arc radius  $20h$  & +/- 45 degree perp to wall)  
 Effective shielding length has been assessed as  $180$  m ( $20 * \text{Building Height}$ )  
 Effective shielding buildings are required to have  $H_t > 9$  m (Shielding Bldgs  $H_t \geq \text{Bldg } H_t$ )  
 (Note if Upwind Slope  $> 0.2$  then no shielding is present - Cl 4.3.1)

Enter	N	NE	E	SE	S	SW	W	NW
No. Bldgs in shielding zone of $h_t > h$	0	0	0	0	0	0	0	0
Avg. Height of shielding bldgs								
Avg. Breadth of shielding bldgs								
$L_s =$	0	0	0	0	0	0	0	0
--	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

**Design Management Consultants**

Page No.: 2

JOB NAME: B & C Morton-Jones House

Date: 12/6/11

Job/File Number: 9000

Designer: Lyall Green

Description: Wind speed calcs

Checker: 1

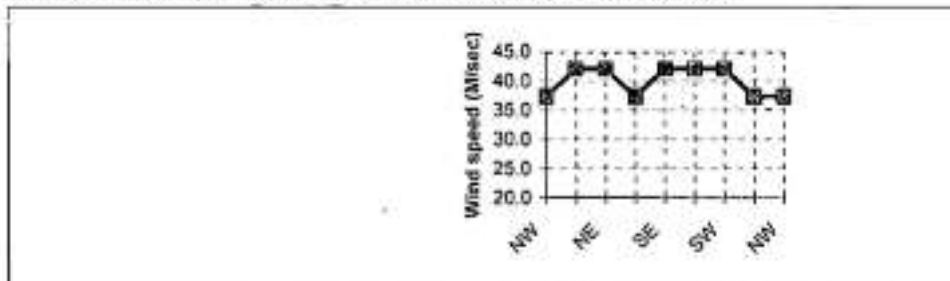
**Terrain Multiplier, Mt**

(Max of Mt, Mo, Mh)

Hill Shape Multiplier	F	F	F	F	F	F	F	F
Enter Upwind Hill Shape (E,H,R,F)	H	H	H	H	H	H	H	E
(E=Escarpment, H=Hill, R=Ridge, F=Flat)								
Ridge Elevation m	12.5	12.5	12.5	12.5	12.5	12.5	12.5	12.5
Upwind Valley Elevation m	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2.5
H = Height of Terrain feature m	10	10	10	10	10	10	10	10
Contour at midheight of feature m								
Enter Lu	74	74	74	74	74	74	150	150
Crest Slope Rad								
Distance from Site to Crest m	30	30	30	30	30	30	30	30
Enter L if site is on Lee side of the slope								
Hill Shape Multiplier Mh	1.058	1.06	1.06	1.06	1.06	1.06	1.00	1.00
Lee Zone Multiplier	N	NE	E	N	S	SW	W	N
Site is within a Lee Zone? (Y or N)				N				N
Distance Site to Leeward Crest km				0.03				0.03
	Nil	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Elevation Multiplier (Eqn 4.4(1)) (Note: 50)								
Enter approx. Elevation if >500 m m	0							
				Then Me =				
Topographic Multiplier Mt	1.06	1.06	1.06	1.06	1.06	1.06	1.00	1.00

	NW	N	NE	E	SE	S	SW	W	NW
Product of Multipliers =		0.93	0.93	0.83	0.93	0.93	0.93	0.83	0.83
Directional Wind Speeds Are	37.3	42.1	42.1	37.4	42.1	42.1	42.1	37.3	37.3

(Note Ultimate Wind Speed shall be greater than 30 m/sec for Ultimate conditions C/ 2.3)



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JOB No :	9006	PAGE :	1
PROJECT :	B & C Morton-Jones House Extensions - Bed 2 Poles	DATE :	14-Feb-08
		BY :	LG

## PILE FOUNDATION DESIGN USING 'BROHMS FOR CLAY' ANALYSES

$$H_u = 9 \cdot C_u \cdot D_s \cdot \left( \sqrt{2 \cdot ((e+L)^2 + (e+fo)^2)} - (L + 2e + fo) \right)$$

$$b = \sqrt{2 \cdot ((e+L)^2 + (e')^2)} - (L + e + e')$$

Soil parameters

$$H_u = 3.6$$

undrained cohesion

$$C_u = 70 \text{ kPa}$$

Pile design actions

lateral force

$$H_u = 3.6 \text{ kN}$$

eccentricity

$$e = 3.90 \text{ m}$$

Trial pile diameter

$$D_s = 400 \text{ mm}$$

Pile embedment Calculation

$$b = 0.014 \quad H_u / (9 \cdot C_u \cdot D_s)$$

$$e' = 4.50 \text{ m} \quad e + 1.5 \cdot D_s \quad e + fo$$

$$l' = 0.52 \text{ m}$$

Design Selection:

Pile embedment length	L =	1.12 m
Pile diameter	Ø =	400 mm

OR Fixed Head Short Piles

$$L = 1.5D + P / (9 \cdot C_u \cdot D)$$

$$L = 0.61 \text{ m}$$

$$M^*_{pile} = 2.19 \text{ kN-m}$$

# Design Management Consultants Ltd



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JOB No :	9006	PAGE :	1
PROJECT :	B & C Morton-Jones House Extensions - Bed 3 Poles	DATE :	14-Feb-08
		BY :	LG

## PILE FOUNDATION DESIGN USING 'BROHMS FOR CLAY' ANALYSES

$$H_u = 9 \cdot C_u \cdot D_s \cdot \left( \sqrt{2 \cdot ((e+L)^2 + (e+fo)^2)} - (L + 2e + fo) \right)$$

$$b = \frac{H_u}{9 \cdot C_u \cdot D_s} \cdot \left( \sqrt{2 \cdot ((e+L)^2 + (e')^2)} - (L + e + e') \right)$$

Soil parameters

$$H_u = 0.2$$

undrained cohesion

$$C_u = 70 \text{ kPa}$$

### Pile design actions

lateral force

$$H_u = 0.2 \text{ kN}$$

eccentricity

$$e = 3.30 \text{ m}$$

trial pile diameter

$$D_s = 400 \text{ mm}$$

### Pile embedment Calculation

$$b = 0.001$$

$$e' = 3.90 \text{ m}$$

$$L' = 0.11 \text{ m}$$

$$H_u / (9 \cdot C_u \cdot D_s)$$

$$e + 1.5 \cdot D_s$$

OR Fixed Head Short Piles

$$L = 1.5D + P / (9 \cdot C_u \cdot D)$$

$$0.60 \text{ m}$$

$$M^*_{pile} = 0.12 \text{ kN-m}$$

### Design Selection

Pile embedment length	$L =$	0.71 m
Pile diameter	$\phi =$	400 mm



# Design Management Consultants Ltd



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Limited

JOB No :	9006	PAGE :	1
PROJECT :	B & C Morton-Jones House Extensions - Bed 3 Poles	DATE :	14-Feb-08
		BY :	LG

## PILE FOUNDATION DESIGN USING 'BROHMS FOR CLAY' ANALYSES

$$H_u = 9 \cdot C_u \cdot D_s \cdot \left( \sqrt{2 \cdot ((e+L)^2 + (e+fo)^2)} - (L + 2e + fo) \right)$$

$$b = \frac{H_u}{9 \cdot C_u \cdot D_s} = \sqrt{2 \cdot ((e+L)^2 + (e+fo)^2)} - (L + 2e + fo)$$

Soil parameters

$$H_u = 6.4$$

undrained cohesion

$$C_u = 70 \text{ kPa}$$

Pile design actions

Internal force	$H_u = 6.4 \text{ kN}$
eccentricity	$e = 0.70 \text{ m}$
Trial pile diameter	$D_{s,d} = 400 \text{ mm}$

Pile embedment Calculation

$b = 0.025$	$H_u / (9 \cdot C_u \cdot D_s)$
$e' = 1.30 \text{ m}$	$e + 1.5 \cdot D_s$
$L' = 0.39 \text{ m}$	$e + fo$

Design Selection

Pile embedment length	$L = 0.99 \text{ m}$
Pile diameter	$\phi = 400 \text{ mm}$

OR Fixed Head Short Piles

$$L = 1.5D + P / (9 \cdot C_u \cdot D)$$

$$L = 0.63 \text{ m}$$

$$M^*_{pile} = 3.92 \text{ kN-m}$$

Job No: 9006  
Project: B & C Morton-Jones House extensions

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Beam B1  
Beam Span: 3.4 m  
Lay: 1 m  
L<sub>1/400</sub>: 1 m

UQL LOAD

G = 0.20 kPa  
Q = 0.25 kPa  
Wu = 0.32 kPa  
Roof Trib width = 1.75 m  
Deck trib width = 0.5 m

POINT LOAD

G = kN  
Q = kN  
Wu = kN  
dist x = m

APPLIED ACTIONS

G = 0.99 kNm  
Q = 1.44 kNm  
Wu = 0.96 kNm  
1.4G = 1.10 kNm  
1.2G + 1.5Q = 3.34 kNm  
G + Wu = 1.55 kNm wind downward  
M<sub>1.4G</sub> = 1.59 kNm  
M<sub>1.2G+1.5Q</sub> = 4.82 kNm  
M<sub>G+Wu</sub> = 2.23 kNm  
V<sub>1.4G</sub> = 1.87 kN  
V<sub>1.2G+1.5Q</sub> = 5.68 kN  
V<sub>G+Wu</sub> = 2.63 kN

1.4G = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4G</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>G+Wu</sub> = 0.00 kNm

TRIAL SIZE

Depth = 190 mm  
Width = 90 mm  
self weight = 0.03 kNm/m  
11400 mm<sup>2</sup>  
51 E6 mm<sup>4</sup>  
342 E3 mm<sup>3</sup>

Grade = MSG 8 - Dry Condition (m/c < 16%)  
f<sub>b</sub> = 14.00 MPa  
f<sub>s</sub> = 3.60 MPa  
f<sub>p</sub> = 8.90 MPa  
E = 8.00 GPa

ULS FLEXURE DESIGN

1.2G + 1.5Q  
k1 = 0.8  
k4 = 1.14  
k5 = 1  
k8 = 1  
M\* = 4.82 kNm  
O/B = 2.1  
S = L<sub>1/400</sub> = 11.11  
aM = 5.53 kNm  
1.4G  
k1 = 0.6  
M\* = 1.59 kNm  
aM = 4.15 kNm

SHEAR DESIGN

SHEAR STRENGTH

1.4G V\* = 1.87 kN  
1.2G+1.5Q V\* = 5.68 kN  
0.9G + Wu V\* = 2.63 kN  
aV<sub>s</sub> = 21.07 kN

BEARING STRESS

A<sub>p</sub> = 9100 mm<sup>2</sup>  
k3 = 1.06 Table 2.6  
aV<sub>rb</sub> = 48.91 kN

SLS DEFLECTION CHECK

G Deflection = 4.17 mm  
Q Deflection = 6.06 mm  
Wu Deflection = 2.37 mm  
Short term G + Qs y = 0.7  
k2 = 1  
D = 8.42 mm  
Short term G + Ws y = 1  
k2 = 1  
D = 5.71 mm  
L/400 = 8.50 mm

Long term G + Qs y = 0  
k2 = 2  
D = 8.33 mm  
L/300 = 11.33 mm

190 x 90 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 16%)

Job No:	9008	PAGE:	1
Project:	B & C McIlroy-Jones House extensions	DATE:	14-Apr-15
	Roof beams	BY:	LG

Beam Identify:	Beam B2 - supports B1
Beam Span:	1.2 m
Lay =	1 m
Lay <sub>max</sub> =	1 m

UDL LOAD		POINT LOAD	
G =	0.40 kPa	G =	1.40 kN
Q =	0.25 kPa	Q =	0.80 kN
Wu =	0.35 kPa	Wu =	1.00 kN
Trb width =	0.50 m	dist x =	0.30 m

APPLIED ACTIONS			
G =	0.28 kN/m		
Q =	0.13 kN/m		
Wu =	0.18 kN/m		
1.4G =	0.37 kN/m	1.4G =	1.95 kN
1.2G + 1.5Q =	0.50 kN/m	1.2G + 1.5Q =	2.88 kN
G + Wu =	0.38 kN/m	0.9G + Wu =	2.28 kN
M <sub>1.4G</sub> =	0.51 kNm	M <sub>1.4G</sub> =	0.44 kNm
M <sub>1.2G+1.5Q</sub> =	0.74 kNm	M <sub>1.2G+1.5Q</sub> =	0.85 kNm
M <sub>G+Wu</sub> =	0.58 kNm	M <sub>0.9G+Wu</sub> =	0.51 kNm
V <sub>1.4G</sub> =	1.59 kN		
V <sub>1.2G+1.5Q</sub> =	2.46 kN		
V <sub>G+Wu</sub> =	1.92 kN		

wind downward

TRIAL SIZE		Grade = MSG 8 - Dry Condition (m/c < 16%)	
Depth	140 mm	f <sub>b</sub> =	14.00 MPa
Width	80 mm	f <sub>a</sub> =	3.80 MPa
self weight:	0.26 kN/m	f <sub>p</sub> =	8.50 MPa
	8400 mm <sup>2</sup>	E =	8.00 GPa
	21 69 mm <sup>4</sup>		
	294 63 mm <sup>3</sup>		

ULS FLEXURE DESIGN			
1.2G + 1.5Q	φ =	0.8	
	k <sub>1</sub> =	0.8	
	k <sub>2</sub> =	1.14	
	k <sub>3</sub> =	1	
	k <sub>4</sub> =	1	
	M* =	0.74 kNm	
	D/S =	1.8	
	S = L <sub>eff</sub> /B	11.11	
	σ <sub>M</sub> =	3.00 kNm	
1.4G	k <sub>1</sub>	0.6	
	M*	0.51 kNm	
	σ <sub>M</sub>	2.25 kNm	

SHEAR DESIGN			
SHEAR STRENGTH			
1.4G	V* =	1.69 kN	
1.2G+1.5Q	V* =	2.46 kN	
0.9G + Wu	V* =	1.92 kN	
	σ <sub>V</sub> =	15.53 kN	

BEARING STRESS			
	A <sub>p</sub> =	8100 mm <sup>2</sup>	
	k <sub>3</sub> =	1.08 Table 2.8	
	σ <sub>Np</sub> =	48.91 kN	

SLS DEFLECTION CHECK			
	G Deflection =	0.35 mm	
	Q Deflection =	0.20 mm	
	Wu Deflection =	0.25 mm	
Short term G + Qs	y =	0.7	
	k <sub>2</sub> =	1	
	D =	0.49 mm	
Short term G + Wu	y =	1	
	k <sub>2</sub> =	1	
	D =	0.51 mm	
	L/300 =	3.00 mm	
Long term G + Qs	y =	0	
	k <sub>2</sub> =	2	
	D =	0.70 mm	
	L/300 =	4.00 mm	

Use 140 x 80 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 16%)

# Design Management Consultants Ltd



Job No: 9006  
Project: B & C Milton Jones House extensions  
Roof beams

PAGE: 1  
DATE: 14-Apr-15  
BY: JG

Beam identity: Beam B3  
Beam Span: 4.4 m  
Lay = 1 m  
Lay<sub>max</sub> = 1 m

**UDL LOAD**  
G = 0.40 kPa  
Q = 0.25 kPa  
Wu = 0.38 kPa  
Trib width = 2.40 m

**POINT LOAD**  
G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0 m

### APPLIED ACTIONS

G = 1.07 kN/m  
Q = 0.60 kN/m  
Wu = 0.90 kN/m  
1.4G = 1.50 kN/m  
1.2G + 1.5Q = 2.18 kN/m  
G + Wu = 1.09 kN/m  
M<sub>1.4G</sub> = 3.62 kNm  
M<sub>1.2G+1.5Q</sub> = 5.28 kNm  
M<sub>G+Wu</sub> = 4.50 kNm

wind downward

1.4Q = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4Q</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>0.9G+Wu</sub> = 0.00 kNm

V<sub>1.4G</sub> = 3.29 kN  
V<sub>1.2G+1.5Q</sub> = 4.80 kN  
V<sub>0.9G+Wu</sub> = 4.09 kN

### TRIAL SIZE

Depth = 240 mm  
Width = 90 mm  
self weight = 0.11 kN/m  
14400 mm<sup>2</sup>  
104 E6 mm<sup>4</sup>  
864 E3 mm<sup>3</sup>

Grade = MSC 8 - Dry Condition (mo < 16%)  
f<sub>t</sub> = 14.00 MPa  
f<sub>c</sub> = 3.90 MPa  
f<sub>p</sub> = 8.90 MPa  
E = 8.00 GPa

### ULS FLEXURE DESIGN

1.2G + 1.5Q  
k<sub>1</sub> = 0.8  
k<sub>2</sub> = 1.14  
k<sub>3</sub> = 1.2  
k<sub>4</sub> = 1.2  
M\* = 5.28 kNm  
D/S = 2.7  
S = LwB = 11.11  
aMn = 8.83 kNm

1.4Q  
k<sub>1</sub> = 0.6  
M\* = 3.62 kNm  
aM = 6.82 kNm

### SHEAR DESIGN

#### SHEAR STRENGTH

1.4Q V\* = 3.29 kN  
1.2G+1.5Q V\* = 4.80 kN  
0.9G + Wu V\* = 4.09 kN

#### BEARING STRESS

aVn = 26.82 kN  
Ap = 8100 mm<sup>2</sup>  
k<sub>3</sub> = 1.06 Table 2.8  
aHnb = 48.91 kN

### SLS DEFLECTION CHECK

G Deflection = 8.28 mm  
Q Deflection = 3.53 mm  
Wu Deflection = 5.30 mm

Short term G + Qs y = 0.7  
k<sub>2</sub> = 1  
D = 8.76 mm

Short term G + Ws y = 1  
k<sub>2</sub> = 1  
D = 9.73 mm  
L/400 = 11.00 mm

Long term G + Qs y = 0  
k<sub>2</sub> = 2  
D = 12.57 mm  
L/300 = 14.67 mm

Use 240 x 90 Timber Beam  
Grade MSC 8 - Dry Condition (mo < 16%)

# Design Management Consultants Ltd



Job No: 9005  
Project: B & C Morlan-Jones House extensions  
Rcaf beams

PAGE: 1  
DATE: 14/07/15  
BY: LGH

Beam Identfy: Beam B4 - supports B3  
Beam Span: 1 m  
Lay = 1 m  
L<sub>eff</sub> = 1 m

### UDL LOAD

G = 0.45 kPa  
Q = 0.25 kPa  
Wu = 0.35 kPa  
Trib width = 0.10 m

### POINT LOAD

G = 2.35 kN  
Q = 1.50 kN  
Wu = 2.00 kN  
dist x = 0.50 m

### APPLIED ACTIONS

G = 0.10 kNm  
Q = 0.03 kNm  
Wu = 0.04 kNm  
1.4G = 0.14 kNm  
1.2G + 1.5Q = 0.18 kNm  
G + Wu = 0.08 kNm  
M<sub>1.4G</sub> = 0.71 kNm  
M<sub>1.2G+1.5Q</sub> = 1.00 kNm  
M<sub>G+Wu</sub> = 0.87 kNm

wind downward

1.4G = 3.29 kN  
1.2G + 1.5Q = 5.07 kN  
0.9G + Wu = 4.12 kN  
M<sub>1.4G</sub> = 0.69 kNm  
M<sub>1.2G+1.5Q</sub> = 1.06 kNm  
M<sub>0.9G+Wu</sub> = 0.86 kNm

V<sub>1.4G</sub> = 2.38 kN  
V<sub>1.2G+1.5Q</sub> = 3.63 kN  
V<sub>0.9G+Wu</sub> = 2.92 kN

### TRIAL SIZE

Depth = 140 mm  
Width = 90 mm  
wet weight = 0.05 kNm  
8400 mm<sup>2</sup>  
21 85 mm<sup>2</sup>  
294 E5 mm<sup>2</sup>

Grade = MSG 8 - Dry Condition (m/c < 16%)

f<sub>b</sub> = 14.00 MPa  
f<sub>v</sub> = 3.80 MPa  
f<sub>p</sub> = 8.90 MPa  
E = 8.00 GPa

### ULS FLEXURE DESIGN

1.2G + 1.5Q  
k1 = 0.8  
k2 = 1.14  
k3 = 1  
k4 = 1  
M\* = 1.06 kNm  
D/S = 1.5  
S = Le/B = 11.11  
aMn = 3.00 kNm

1.4G  
k1 = 0.6  
M\* = 0.71 kNm  
aV = 2.35 kNm

### SHEAR DESIGN

#### SHEAR STRENGTH

1.4G V\* = 2.38 kN  
1.2G+1.5Q V\* = 3.63 kN  
0.9G + Wu V\* = 2.92 kN

#### BEARING STRESS

aVs = 15.53 kN  
Ap = 8100 mm<sup>2</sup>  
k3 = 1.05 Table 2.8  
aHnb = 48.61 kN

### SLS DEFLECTION CHECK

G Deflection = 0.31 mm  
Q Deflection = 0.19 mm  
Wu Deflection = 0.26 mm  
Short term G + Qs y = 0.7  
k2 = 1  
D = 0.44 mm  
Short term G + Ws y = 1  
k2 = 1  
D = 0.47 mm  
LH00 = 2.50 mm

#### Long term G + Qs

y = 0  
k2 = 2  
D = 0.61 mm  
L/300 = 3.33 mm

Use 140 x 90 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 16%)

# Design Management Consultants Ltd



Job No: 8005  
Project: B & C-Morton Jones House extensions  
Roof beams

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Beam B5 - supports veranda  
Beam Span: 3 m  
Lay = 1 m  
Lay<sub>width</sub> = 1 m

**UDL LOAD**  
G = 0.40 kPa  
Q = 0.25 kPa  
Wu = 0.51 kPa wind downward  
Trib width = 0.60 m

**POINT LOAD**  
G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0.00 m

**APPLIED ACTIONS**  
G = 0.28 kNm  
Q = 0.15 kNm  
Wu = 0.33 kNm  
1.4G = 0.40 kNm  
1.2G + 1.5Q = 0.55 kNm  
C + Wu = 0.55 kNm wind downward  
M<sub>1.4G</sub> = 0.45 kNm  
M<sub>1.2G+1.5Q</sub> = 0.43 kNm  
M<sub>0.9G+Wu</sub> = 0.63 kNm  
V<sub>1.4G</sub> = 0.59 kN  
V<sub>1.2G+1.5Q</sub> = 0.65 kN  
V<sub>0.9G+Wu</sub> = 0.64 kN

1.4Q = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4Q</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>0.9G+Wu</sub> = 0.00 kNm

**TRIAL SIZE**  
Depth 190 mm  
Width 45 mm  
self weight 0.04 kNm  
5/99 mm<sup>2</sup>  
26 1.8 mm<sup>2</sup>  
271 E5 mm<sup>2</sup>

Grade = MSG 8 - Dry Condition (mo < 18%)  
fb = 14.00 MPa  
fs = 3.80 MPa  
fp = 8.80 MPa  
E = 8.00 GPa

**ULS FLEXURE DESIGN**  
a = 0.8  
1.2G + 1.6Q  
k1 = 0.6  
k4 = 1  
k5 = 1  
k6 = 1  
M\* = 0.63 kNm  
D/B = 4.2  
S = Le/B = 22.22  
aMn = 2.43 kNm  
1.4Q  
k1 = 0.6  
M\* = 0.45 kNm  
aMn = 1.82 kNm

**SHEAR DESIGN**  
**SHEAR STRENGTH**  
1.4Q V\* = 0.59 kN  
1.2G+1.5Q V\* = 0.65 kN  
0.9G + Wu V\* = 0.64 kN  
aVn = 9.24 kN  
**BEARING STRESS**  
Ap = 4050 mm<sup>2</sup>  
k3 = 1.00 Table 2.6  
aNb = 24.45 kN

**SLS DEFLECTION CHECK**  
G Deflection = 1.45 mm  
Q Deflection = 0.77 mm  
Wu Deflection = 1.60 mm  
Short term G + Qs y = 0.7  
k2 = 1  
Q = 1.98 mm  
Short term G + Wa y = 1  
k2 = 1  
Q = 2.51 mm  
L/400 = 7.50 mm  
Long term G + Qs y = 0  
k2 = 1/2  
Q = 2.90 mm  
L/300 = 10.00 mm

Use 190 x 45 Timber Beam  
Grade MSG 8 - Dry Condition (mo < 18%)

# Design Management Consultants Ltd



Job No: 9006  
Project: B & C Morton-Jones House extensions  
Floor Beam

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Bed 2 floor beam A  
Beam Span: 2.7 m  
Lay = 1 m  
Lay width = 1 m

### UDL LOAD

G = 1.40 kPa  
Q = 1.50 kPa  
Wu = 0.00 kPa  
Trib width = 2.10 m

### POINT LOAD

G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0.00 m

### APPLIED ACTIONS

G = 0.87 kNm  
Q = 3.24 kNm  
Wu = 0.00 kNm  
1.4G = 1.58 kNm  
1.2G + 1.5Q = 6.03 kNm  
G + Wu = 0.86 kNm  
M<sub>1.4G</sub> = 1.24 kNm  
M<sub>1.2G+1.5Q</sub> = 5.49 kNm  
M<sub>G+Wu</sub> = 0.79 kNm  
V<sub>1.4G</sub> = 1.84 kN  
V<sub>1.2G+1.5Q</sub> = 8.14 kN  
V<sub>G+Wu</sub> = 1.17 kN

1.4G = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4G</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>G+Wu</sub> = 0.00 kNm

of 4.87 in Microstran

### TRIAL SIZE

Depth: 240 mm  
Width: 90 mm  
self weight: 0.11 kNm  
14400 mm<sup>2</sup>  
104 E5 mm<sup>2</sup>  
864 E3 mm<sup>2</sup>

Grade = MSG 8 - Dry Condition (m/c < 16%)

f<sub>b</sub> = 14.00 MPa  
f<sub>c</sub> = 3.80 MPa  
f<sub>p</sub> = 8.90 MPa  
E = 8.00 GPa

### ULS FLEXURE DESIGN

1.2G + 1.5Q  
a = 0.8  
k<sub>1</sub> = 0.8  
k<sub>4</sub> = 1.14  
k<sub>5</sub> = 1  
k<sub>8</sub> = 1  
M\* = 5.49 kNm  
D/S = 2.7  
S = L<sub>eff</sub>/B = 11.11  
aM<sub>u</sub> = 8.83 kNm  
1.4G  
k<sub>1</sub> = 0.8  
M\* = 1.24 kNm  
aV = 8.82 kNm

### SHEAR DESIGN

#### SHEAR STRENGTH

1.4G V\* = 1.84 kN  
1.2G+1.5Q V\* = 8.14 kN  
0.9G + Wu V\* = 1.17 kN

#### BEARING STRESS

aV<sub>u</sub> = 28.82 kN  
A<sub>p</sub> = 8100 mm<sup>2</sup>  
k<sub>3</sub> = 1.05 Table 2.6  
aM<sub>u</sub> = 48.91 kN

### SLS DEFLECTION CHECK

G Deflection = 0.81 mm  
Q Deflection = 2.70 mm  
Wu Deflection = 0.00 mm  
Short term G + Qs  
y = 0.7  
k<sub>2</sub> = 1  
D = 2.70 mm  
Short term G + Wa  
y = 1  
k<sub>2</sub> = 1  
D = 0.81 mm  
L/400 = 6.75 mm

#### Long term G + Qs

y = 0  
k<sub>2</sub> = 2  
D = 1.62 mm  
L/300 = 9.00 mm

Use 240 x 90 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 16%)

# Design Management Consultants Ltd



Job No: 9008  
Project: B & C Morton-Jones House extensions  
Fibre beam

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Bed 2 floor joists  
Beam Span: 2.7 m  
Lay = 1 m  
Lay<sub>vert</sub> = 1 m

**UDL LOAD**  
G = 0.40 kPa  
Q = 1.50 kPa  
Wu = 0.00 kPa  
Trib width = 0.40 m

**POINT LOAD**  
G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0.00 m

**APPLIED ACTIONS**

G =	0.19 kN/m	
Q =	0.60 kN/m	
Wu =	0.00 kN/m	
1.4G =	0.27 kN/m	
1.2G + 1.5Q =	1.13 kN/m	
G + Wu =	0.16 kN/m	wind downward
M <sub>1.4G</sub> =	0.24 kNm	
M <sub>1.2G+1.5Q</sub> =	1.03 kNm	
M <sub>G+Wu</sub> =	0.15 kNm	
V <sub>1.4G</sub> =	0.36 kN	
V <sub>1.2G+1.5Q</sub> =	1.53 kN	
V <sub>G+Wu</sub> =	0.22 kN	

**1.4G = 0.00 kN**  
**1.2G + 1.5Q = 0.00 kN**  
**0.9G + Wu = 0.00 kN**  
**M<sub>1.4G</sub> = 0.00 kNm**  
**M<sub>1.2G+1.5Q</sub> = 0.00 kNm**  
**M<sub>G+Wu</sub> = 0.00 kNm**

**TRIAL SIZE**

Depth	140 mm	Grade = MSG 8 - Dry Condition (mo < 16%)
Width	45 mm	f <sub>b</sub> = 14.00 MPa
Self weight	0.03 kN/m	f <sub>s</sub> = 3.80 MPa
	4200 mm <sup>2</sup>	f <sub>p</sub> = 8.90 MPa
	10 E6 mm <sup>4</sup>	E = 8.00 GPa
	147 E3 mm <sup>3</sup>	

**ULS FLEXURE DESIGN**

1.2G + 1.5Q  
k<sub>1</sub> = 0.8  
k<sub>4</sub> = 1  
k<sub>5</sub> = 1  
k<sub>6</sub> = 1  
M\* = 1.03 kNm  
D/S = 3.1  
S = LevB  
aMn = 22.22  
aMn = 1.32 kNm

1.4G  
k<sub>1</sub> = 0.8  
M\* = 0.24 kNm  
aV = 0.59 kNm

**SHEAR DESIGN**

**SHEAR STRENGTH**  
1.4G V\* = 0.36 kN  
1.2G+1.5Q V\* = 1.53 kN  
0.9G + Wu V\* = 0.22 kN  
aVn = 6.91 kN

**BEARING STRESS**  
A<sub>p</sub> = 4050 mm<sup>2</sup>  
k<sub>3</sub> = 1.06 Table 2.6  
aNnb = 24.45 kN

**SLS DEFLECTION CHECK**

G Deflection =	1.51 mm	Long term G + Qs	y = 0
Q Deflection =	5.04 mm	k <sub>2</sub> = 2	
Wu Deflection =	0.00 mm	D = 3.22 mm	
Short term G + Qs	y = 0.7	L/300 = 9.00 mm	
k <sub>2</sub> = 1			
D = 5.14 mm			
Short term G + Wu	y = 1		
k <sub>2</sub> = 1			
D = 1.61 mm			
L/400 = 6.75 mm			

Use 140 x 45 Timber Beam  
Grade MSG 8 - Dry Condition (mo < 16%)



# Design Management Consultants Ltd



Job No: 9008  
Project: B & C Morley-Jones House extensions  
Floor beam

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Bed 2 floor joists  
Beam Span: 3.1 m  
Lay = 1 m  
Lay<sub>sum</sub> = 1 m

### UQL LOAD

G = 0.40 kPa  
Q = 1.50 kPa  
Wu = 0.00 kPa  
Tri<sub>width</sub> = 0.40 m

### POINT LOAD

G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0.00 m

### APPLIED ACTIONS

G = 0.20 kN/m  
Q = 0.60 kN/m  
Wu = 0.00 kN/m  
1.4G = 0.28 kN/m  
1.2G + 1.5Q = 1.14 kN/m  
G + Wu = 0.16 kN/m wind downward  
M<sub>1.4G</sub> = 0.34 kNm  
M<sub>1.2G+1.5Q</sub> = 1.37 kNm  
M<sub>G+Wu</sub> = 0.19 kNm  
V<sub>1.4G</sub> = 0.44 kN  
V<sub>1.2G+1.5Q</sub> = 1.77 kN  
V<sub>G+Wu</sub> = 0.25 kN

1.4Q = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4Q</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>0.9G+Wu</sub> = 0.00 kNm

### TRIAL SIZE

Depth = 193 mm  
Width = 45 mm  
self weight = 0.04 kN/m  
6700 mm<sup>2</sup>  
26 E6 mm<sup>2</sup>  
271 E3 mm<sup>2</sup>

Grade = MSG 8 - Dry Condition (m/c < 16%)

f<sub>b</sub> = 14.00 MPa  
f<sub>s</sub> = 3.80 MPa  
f<sub>p</sub> = 8.90 MPa  
E = 8.00 GPa

### SLS FLEXURE DESIGN

1.2G + 1.5Q  
k<sub>1</sub> = 0.8  
k<sub>4</sub> = 1  
k<sub>5</sub> = 1  
k<sub>8</sub> = 1  
M\* = 1.37 kNm  
D/S = 4.2  
S = Lw<sub>0</sub> = 22.22  
a<sub>1k</sub> = 2.43 kNm  
1.4G  
k<sub>1</sub> = 0.8  
M\* = 0.34 kNm  
a<sub>1k</sub> = 1.82 kNm

### SHEAR DESIGN

#### SHEAR STRENGTH

1.4G V\* = 0.44 kN  
1.2G+1.5Q V\* = 1.77 kN  
0.9G + Wu V\* = 0.25 kN

a<sub>1k</sub> = 9.24 kN

#### BEARING STRESS

A<sub>p</sub> = 4050 mm<sup>2</sup>  
k<sub>3</sub> = 1.05 Table 2.6  
a<sub>Nk3</sub> = 24.45 kN

### SLS DEFLECTION CHECK

G Deflection = 1.18 mm  
Q Deflection = 3.51 mm  
Wu Deflection = 0.00 mm  
Short term G + Qs  
γ = 0.7  
k<sub>2</sub> = 1  
Q = 3.64 mm  
Short term G + Wu  
γ = 1  
k<sub>2</sub> = 1  
Q = 1.18 mm  
L/400 = 7.75 mm

#### Long term G + Qs

γ = 0  
k<sub>2</sub> = 2  
Q = 3.37 mm  
L/300 = 10.33 mm

Use 193 x 45 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 16%)

Job No: 9008  
Project: B & C Morton-Jones House extensions  
Floor Beam

PAGE: 1  
DATE: 14-Apr-15  
BY: LG

Beam Identity: Bed 3 floor beam B  
Beam Span: 3 m  
Lay = 1 m  
Lay<sub>min</sub> = 1 m

**UDL LOAD**  
G = 2.40 kPa  
Q = 1.50 kPa  
Wu = 0.00 kPa  
Trib width = 2.20 m

**POINT LOAD**  
G = 0.00 kN  
Q = 0.00 kN  
Wu = 0.00 kN  
dist x = 0.00 m

**APPLIED ACTIONS**

G = 0.89 kN/m  
Q = 1.30 kN/m  
Wu = 0.00 kN/m  
1.4G = 1.99 kN/m  
1.2G + 1.5Q = 6.14 kN/m  
G + Wu = 0.89 kN/m  
M<sub>1.4G</sub> = 1.55 kNm  
M<sub>1.2G+1.5Q</sub> = 6.90 kNm of 4.87 in Microstran  
M<sub>G+Wu</sub> = 0.95 kNm  
V<sub>1.4G</sub> = 2.07 kN  
V<sub>1.2G+1.5Q</sub> = 8.20 kN  
V<sub>G+Wu</sub> = 1.32 kN

1.4G = 0.00 kN  
1.2G + 1.5Q = 0.00 kN  
0.9G + Wu = 0.00 kN  
M<sub>1.4G</sub> = 0.00 kNm  
M<sub>1.2G+1.5Q</sub> = 0.00 kNm  
M<sub>G+Wu</sub> = 0.00 kNm

**TRIAL SIZE**

Depth = 240 mm  
Width = 90 mm  
Self weight = 0.11 kN/m  
14400 mm<sup>2</sup>  
104 E3 mm<sup>3</sup>  
884 E3 mm<sup>3</sup>

Grade = MSG 8 - Dry Condition (m/c < 18%)

f<sub>b</sub> = 14.00 MPa  
f<sub>a</sub> = 3.80 MPa  
f<sub>p</sub> = 8.50 MPa  
E = 8.00 GPa

**ULS FLEXURE DESIGN**

1.2G + 1.5Q  
α = 0.9  
k<sub>1</sub> = 0.9  
k<sub>2</sub> = 1.14  
k<sub>3</sub> = 1  
k<sub>4</sub> = 1  
M\* = 6.90 kNm of Microstran = 5.8kN-m  
D/B = 2.7  
S = L<sub>eff</sub>/B = 11.11  
αM<sub>1</sub> = 8.83 kNm  
1.4G  
k<sub>1</sub> = 0.5  
M\* = 1.55 kNm  
αV = 6.82 kNm

**SHEAR DESIGN**

**SHEAR STRENGTH**

1.4G V\* = 2.07 kN  
1.2G+1.5Q V\* = 8.20 kN  
0.9G + Wu V\* = 1.32 kN

αV<sub>h</sub> = 26.82 kN

**BEARING STRESS**

A<sub>p</sub> = 8100 mm<sup>2</sup>  
k<sub>3</sub> = 1.05 Table 2.6  
αk<sub>3</sub>b = 48.91 kN

**SLS DEFLECTION CHECK**

G Deflection = 1.25 mm  
Q Deflection = 4.20 mm  
Wu Deflection = 0.00 mm  
Short term G + Qs  
y = 0.7  
k<sub>2</sub> = 1  
D = 4.19 mm  
Short term G + Ws  
y = 1  
k<sub>2</sub> = 1  
D = 1.25 mm  
L/400 = 7.50 mm

**Long term G + Qs**

y = 0  
k<sub>2</sub> = 2  
D = 2.51 mm  
L/300 = 10.00 mm

Use 240 x 90 Timber Beam  
Grade MSG 8 - Dry Condition (m/c < 18%)

Bed 2 Beam Supported on Double stud Posts within existing basement wall framing

Post height = 2.60 m

Loads from Microstran

$1.2G + 1.5Q = 9.30 \text{ kN}$

Use 90\*90 post in basement wall framing

$\phi = 0.80$

$k_1 = 0.80$

$k_6 = 1.14$

$L/D = 28.89 \quad k_8 = 0.32$

$f_c = 15.00$

$\phi N_c = 90 \times 90 \times [0.8 \times 15 \times (k)]$   
28.37 kN

$> N_c^* = 9.30 \text{ kN}$

90\*90 double stud supported on existing  
150mm concrete wall and strip footing Ok



Design Management Consultants  
Limited

9006-2011

30 January 2015

Auckland Council  
Private Bag 92 300  
Auckland 1142

Attention: The Building Inspector

155 Te Rapa Road  
PO Box 5254, Hamilton 3242  
Website www.dmcnz.com  
Email info@dmcnz.com  
Phone +64 7 850 5155  
Fax +64 7 850 5158

Dear Sir / Madam

**RE: GEOTECHNICAL ASSESSMENT FOR HOUSE EXTENSIONS  
B&C MORTON-JONES  
52 KAURI POINT ROAD WAITAKERE**

**A. INTRODUCTION**

Proposed alterations to the existing house are single storey extensions comprising a bedroom extension at the front of the house and a second bedroom at the seaward side of the house as shown on the drawings.

**B. PURPOSE**

A Limited Geotechnical Assessment of the above property was completed to determine the bearing strength of soils supporting existing and new foundations with consideration of other environmental effects.

**C. SOIL TYPE**

The soil type is noted on the GNS Auckland Geological Map as comprising mainly marine sedimentary and volcanic rock with areas noted as clay.

**D. PREVIOUS INVESTIGATIONS**

An earlier geotechnical investigation was carried out for house alterations in 2011. These results shown in our report 9006-2011 dated 16/6/2011 is attached to this report as it summarises the soil type and confirms the construction of the existing deck pole foundations.

The attached results in the reference report are consistent with the additional site geotechnical investigations carried out specifically for the current proposed house extensions.

**E. RESULTS**

The extent of further investigation work carried out on site for the current proposed bedroom extensions included hand auger boreholes, insitu shear vane tests within the clay bearing soils adjacent to each bore hole and measurement of ground profile slopes in the vicinity of the house.

The location of the 4 bore holes is shown for reference on the attached plan.

The soil profile adjacent at bore hole locations generally comprised 100 - 300mm of topsoil over motley orange light brown silt over firm grey clayey silt from 1.0m depth below ground level.

Insitu shear vane soil tests were carried out in the clay soils at varying depths. These shear vane tests included a minimum of three tests at each depth location in order to obtain the remoulded soil shear strength.

The insitu shear vane test results recorded initial shear strengths in the range of 130 to 190kPa which reduced to a minimum reliable remoulded soil shear strength of 70kPa throughout the clayey silts below 1.0m depth from the existing ground surface.

The Ultimate Limit State soil bearing strength within the clayey soils beneath the pole circular foundations is (Terzaghi)  $q_{us} = 1.3 \cdot c \cdot N_c + q_0 \cdot N_q + 0.3B \cdot \gamma \cdot N_\gamma$  or conservatively for soils with zero degree angle of shearing resistance  $q_{us} = 1.3 \cdot c \cdot N_c$  where  $N_c = 5.14$ .

On this basis with  $\phi = 0.5$ , a conservative Ultimate Limit State soil bearing strength within these clay soils for ULS design loading is  $\phi q_{us} (0.5 \cdot 1.3 \cdot 70 \cdot 5.14) = 230 \text{ kPa}$

## F. SLOPE STABILITY

The slopes found on this site are variable with measured slopes within the vicinity of the house varying up to 29 degrees with an average slope over the property of 1:12 (5 degrees). This is typical of slopes within the area.

The site is within the Natural area identified on council plans as Coastal. This area does not contain any sensitive ridge zones identified on council plans.

There are no signs of local instability of the slopes within the vicinity of the existing house

The proposed house alterations will be supported on poles at each borehole location with minimal ground disturbance limited to the immediate vicinity at each pole and thereby the new pole foundations will not reduce the stability of local soils around the house.

The existing storm water discharges through perforated drains distributed throughout the surrounding bush. This system needs to be extended in order to evenly distribute the increased collection of storm water from the additional 38m<sup>2</sup> of bedroom roof area through the bush under growth. The length of the existing perforated drains are to be extended by an additional 60% of the existing comprising 100mm perforated drain located within the topsoil and run parallel to the ground contour at a maximum gradient of 1:200

The full storm water disposal should be constantly maintained to ensure even distribution is maintained and to avoid local scouring of the soils.

## G. FOUNDATIONS

The proposed house alterations require additional poles to support the new bedroom extensions.

Due to upper soft soils and account for existing slopes the new poles need to be concrete encased to a minimum depth of 1.0m below the bottom of soft silts. In the case of poles located at boreholes 1 and 2 the pole foundation shall extend to a minimum of 2m (5m below floor level) and 1.5m (4m below floor level) respectively below ground level.

Poles and associated foundations must be specifically design by a CPEng qualified structural engineer.

### Note:

*The Civil Engineering and Geotechnical opinions and recommendations which are contained in this Report are based on Site conditions as they presently exist and further assume that the exploratory holes and soundings are representative of subsurface conditions throughout the site i.e. Inferences about the nature and continuity of the ground conditions away from the boreholes have been made in the preparation of this report. It is assumed that the subsurface conditions everywhere are not significantly different from those disclosed by the investigation.*

*Actual conditions may vary across the Site however and in some locations may differ from those described herein.*

*We should be notified of any subsurface conditions which appear to be different than those disclosed by this investigation so that these conditions may be reviewed and our recommendations reconsidered where necessary.*

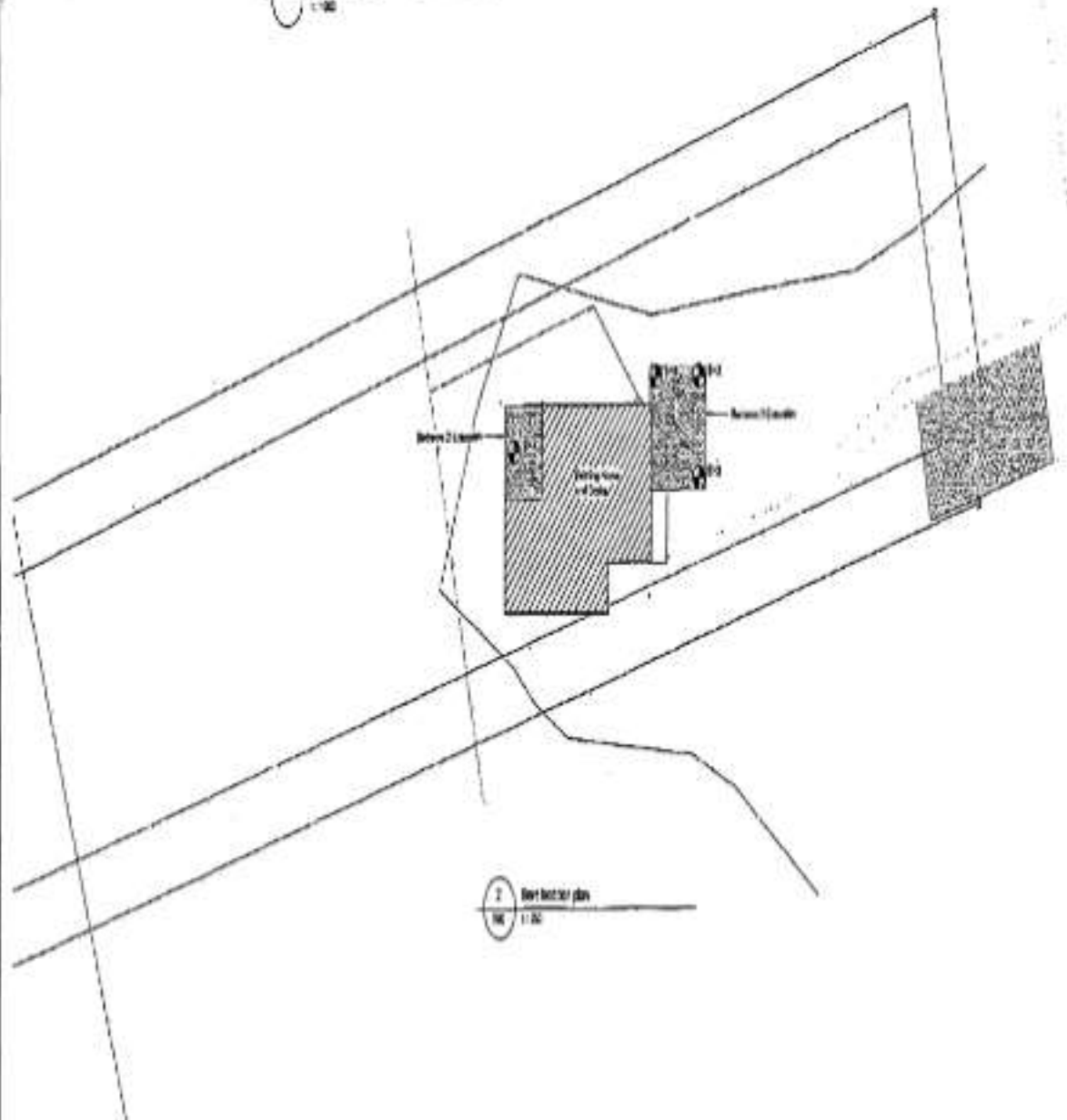
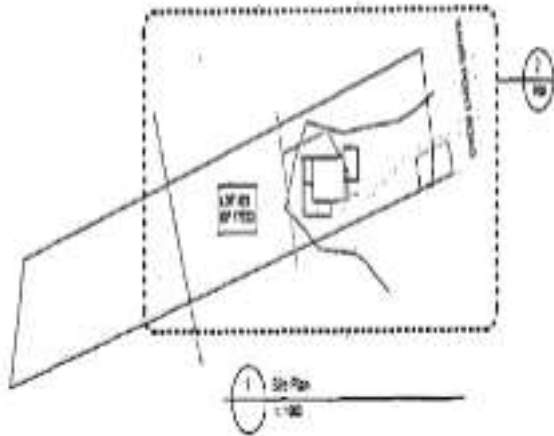
*This Report is limited in nature to the comments contained within the report. The Report has been prepared for use by B & C Morton-Jones and the Auckland Council only. No responsibility will be accepted for its interpretation or use by any other person.*

Yours faithfully  
**DESIGN MANAGEMENT CONSULTANTS LTD**



LYALL GREEN  
BE (Hons) CPEng IntPE # 46696

Encl: Bore hole site plan  
DMC letter dated 16/6/2011



**dmc**  
 Design Management Consultants  
 Limited  
 135 The Regent Road, Northfleet, New Zealand  
 Ph: 64-7-658-0133 Fax: 64-7-658-0138  
 www.dmc.co.nz info@dmc.co.nz

Mr & Mrs Morton-Jones  
 House Extension  
 52 Kauri Point Road,  
 Langitahi

Stone Holes Location Plan

Client	Designer	Arch	Dr	Scale	Imp/permissions	Contract	Drawing	Rev
					Approved	100	R00	
Drawn	Check	By		Date	Project Manager/Consultant			
N.L.	J.P.Z.B.							
Notes				1. This drawing is a site plan showing the location of the stone holes. It is not a structural drawing and should not be used for that purpose.				

Copyright © 2008 DMC

# SOIL DESCRIPTION

## FIELD TEST DATA

BORE HOLE No: 1      3100mm below GFL      SURFACE ELEVATION

Depth in meters	Shear Strength (kPa)	SCALA PENETROMETER (blows per 100mm drop)																		
		1	2	3	4	5	6	7	8	9	10									
0.0																				
0.5																				
1.0	150/95 200/40																			
1.5																				
2.0																				

The stratification lines shown represent the approximate boundary between soil types. The transition may in fact be gradual



Design Management Consultants Limited

Mr & Mrs Morton-Jones  
House Extensions  
52 Mauld Port Road, Llanghelo

Bore Log

SCALE: 1:20	DRAWN: M.Z.	CONTRACT: 8006	
© Copyright: Design Management Consultants Ltd	DATE: Jan 2015	DRAWING: R01	REV.

NOTE: Client to confirm and verify dimensions, extent and levels prior to commencing work.  
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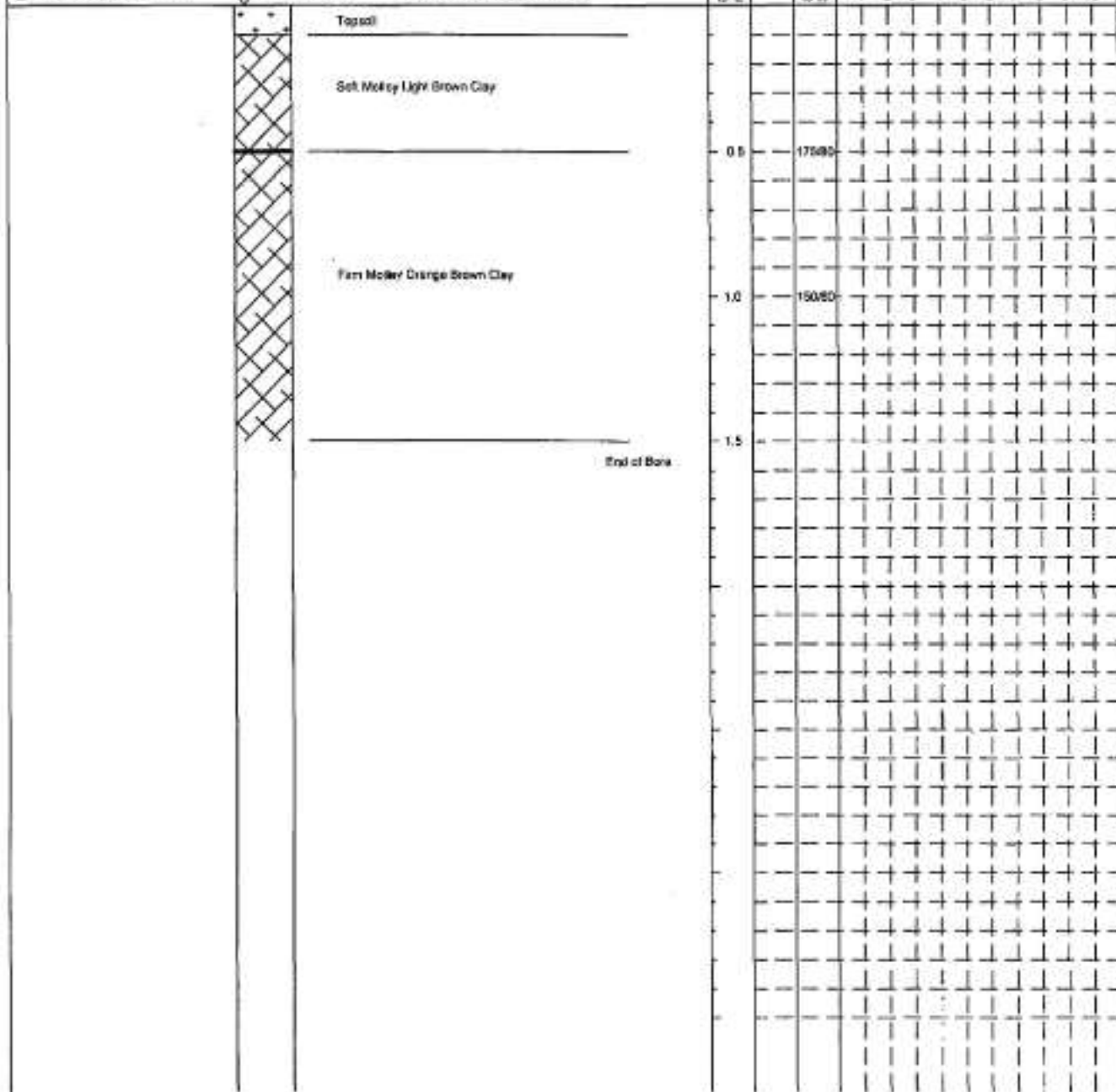
# SOIL DESCRIPTION

## FIELD TEST DATA

BORE HOLE No: 2

2500mm below GFL

SURFACE ELEVATION



The stratification lines shown represent the approximate boundary between soil types. The transition may in fact be gradual.



Design Management Consultants Limited

Mr & Mrs Morton-Jones  
House Extensions  
52 Keel Point Road, Lingshale

Bore Log

SCALE: 1:20	DRAWN: M.Z.	CONTRACT: 9006	
© Copyright Design Management Consultants Ltd	DATE: Jan 2019	DRAWING: R02	REV:

NOTE: Contractor to check and verify all dimensions, levels and levels prior to commencing work.  
 DESIGN: Copyright of this drawing is retained by Design Management Consultants Ltd  
 DRAWN: [Signature]

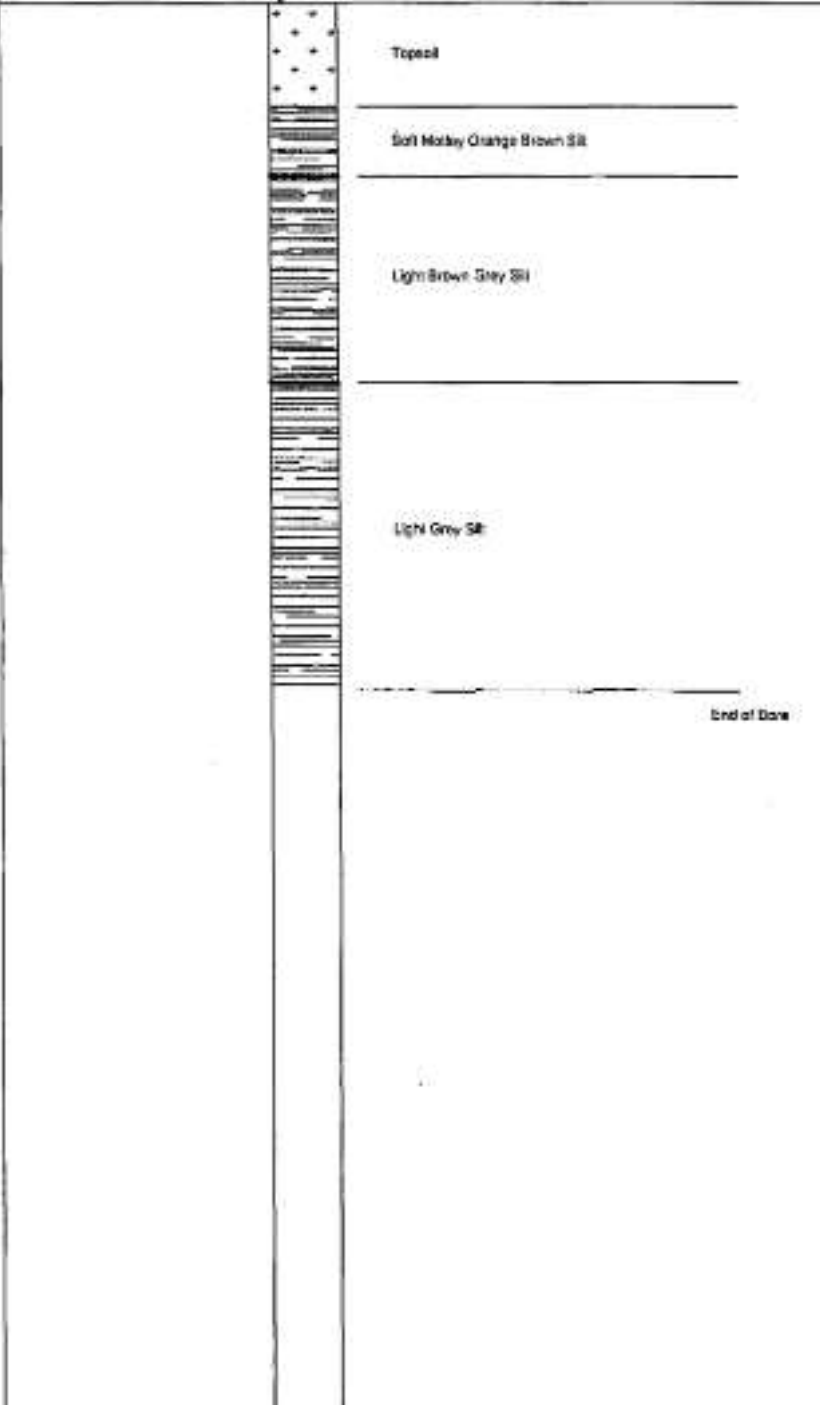
# SOIL DESCRIPTION

## FIELD TEST DATA

BORE HOLE No: 3

GFL + 320mm

SURFACE ELEVATION



Depth in meters	Shear Strength (kPa)	SCALA PENETROMETER (blows per 100mm drop)												
		1	2	3	4	5	6	7	8	9	10			
0.0														
0.5														
1.0	170/80													
1.5	240/120													
2.0														

The stratification lines shown represent the approximate boundary between soil types. The transition may in fact be gradual.



Design Management Consultants Limited

Mr & Mrs Norton-Jones  
House Extensions  
52 Kauri Palm Road, Llangolton

Bore Log

SCALE: 1:20	DRAWN: M.Z.	CONTRACT: 9035	
© Copyright Design Management Consultants Ltd	DATE: Jan 2015	DRAWING: R03	REV:

NOTE: Contractor to check and verify all dimensions, levels and levels prior to commencing work.  
 SCALE: 25. Confirm that this drawing is correct for the work to be carried out. It is the responsibility of the contractor to verify all dimensions and levels on site.

# SOIL DESCRIPTION

## FIELD TEST DATA

BORE HOLE No: 4

2500mm below GFL

SURFACE ELEVATION

Depth in metres	Shear Strength (kPa)	SCALA PENETROMETER (blows per 100mm drop)													
		1	2	3	4	5	6	7	8	9	10				
0.0															
0.5															
1.0															
1.5															
2.0															
2.5															
3.0															
3.5															
4.0															
4.5															
5.0															
5.5															
6.0															
6.5															
7.0															
7.5															
8.0															
8.5															
9.0															
9.5															
10.0															



The stratification lines shown represent the approximate boundary between soil types. The transition may in fact be gradual



Mr & Mrs Worton-Jones  
House Extensions  
52 Kauri Point Road, Leighton

Bore Log

Design Management Consultants Limited

SCALE: 1:20	DRAWN: MZ	CONTRACT: 9006	
Copyright: Design Management Consultants Ltd	DATE: Jan 2010	DRAWING: R04	REV:
<small>NOTES: Core also to be taken and every 100mm, also a wet weight and to be oven-dried at 105°C.</small>			
<small>REMARKS: Ownership of the drawing is retained by Design Management Consultants Ltd until such time as the client has agreed in writing to all drawings issued for any project to proceed.</small>			



Design Management Consultants  
Limited

9006-2011

16 June 2011

Auckland Council  
Private Bag 92 300  
Auckland 1142

Attention: The Building Inspector

155 Te Rapa Road  
PO Box 6254, Hamilton 3242  
Website [www.dmcnz.com](http://www.dmcnz.com)  
Email [info@dmcnz.com](mailto:info@dmcnz.com)  
Phone +64 7 850 5155  
Fax +64 7 850 5158

COPY

Dear Sir / Madam

RE: GEOTECHNICAL ASSESSMENT  
HOUSE ALTERATIONS – B&C MORTON-JONES  
52 KAURI POINT ROAD WAITAKERE

A. INTRODUCTION

Proposed alterations to the existing house will largely be confined to changes to internal partitions apart from an extension to the ground floor level decking as shown on the drawings.

B. PURPOSE

We have carried out a Limited Geotechnical Assessment of the above property to determine the bearing strength of soils supporting existing and new foundations with consideration of other environmental effects.

C. SOIL TYPE

The soil type is noted on the GNS Auckland Geological Map as comprising mainly marine sedimentary and volcanic rock with areas noted as clay.

D. RESULTS

The extent of investigation work carried out on site included excavations to verify the depth of existing concrete and pole foundations were as shown on existing house plans obtained from council records, as well as insitu shear vane tests within the clay bearing soils located at the bottom of the existing footings and pole foundations.

The soil profile adjacent to existing pole foundations comprised 100mm of topsoil over orange-brown pumiceous silty clay to 2.0m depth.

Insitu shear vane soil tests were carried out in the clay soils at varying depths. These shear vane tests included a minimum of three tests at each depth location in order to obtain the remoulded soil shear strength.

The Insitu shear vane test results recorded initial shear strength of 240kPa which reduced to a minimum reliable remoulded soil shear strength of 70kPa throughout the clays within 2m depth from the surface.

The Ultimate Limit State soil bearing strength within the clayey soils beneath the pole circular foundations is (Terzaghi)  $q_{us} = 1.3 \cdot c \cdot N_c + q \cdot N_q + 0.3 B \cdot \gamma \cdot N_\gamma$  or conservatively for soils with zero degree angle of shearing resistance  $q_{us} = 1.3 \cdot c \cdot N_c$  where  $N_c = 5.14$ .

On this basis with  $\phi = 0.5$ , a conservative Ultimate Limit State soil bearing strength within these clayey soils for ULS design loading is  $\phi \cdot c_{us} (0.5 \cdot 1.3 \cdot 70 \cdot 5.14) = 230 \text{ kPa}$

# COPY



Design Management Consultants  
Limited

## E. SLOPE STABILITY

The slopes found on this site are variable with measured slopes within the vicinity of the house varying up to 28 degrees with an average slope over the property of 1:12 (5 degrees). This is typical of slopes within the area.

The site is within the Natural area identified on council plans as Coastal. This area does not contain any sensitive ridge zones identified on council plans.

There are no signs of local instability of the slopes within the vicinity of the existing house

The proposed house alterations do not extend the foot print of the existing house foundations and therefore these house alterations will not influence the stability of local soils around the house.

As the house roof area will not increase, the stormwater run off will remain unchanged. The existing storm water disposal through perforated drains distributed throughout the surrounding bush should be constantly maintained to ensure even distribution occurs to avoid local scouring of the soils.

## F. FOUNDATIONS

The proposed house alterations do not require alteration to the house foundations however decking may be extended at the ground floor level.

Foundations for any new deck extensions are to be installed to minimise any disturbance of the natural habitat by utilising pole piling foundations designed in accordance with NZS 3604 encased in concrete extended to 1m below ground level.

### Note:

*The Civil Engineering and Geotechnical opinions and recommendations which are contained in this Report are based on Site conditions as they presently exist and further assume that the exploratory holes and soundings are representative of subsurface conditions throughout the site i.e. inferences about the nature and continuity of the ground conditions away from the boreholes have been made in the preparation of this report. It is assumed that the subsurface conditions everywhere are not significantly different from those disclosed by the investigation.*

*Actual conditions may vary across the Site however and in some locations may differ from those described herein.*

*We should be notified of any subsurface conditions which appear to be different than those disclosed by this investigation so that these conditions may be reviewed and our recommendations reconsidered where necessary.*

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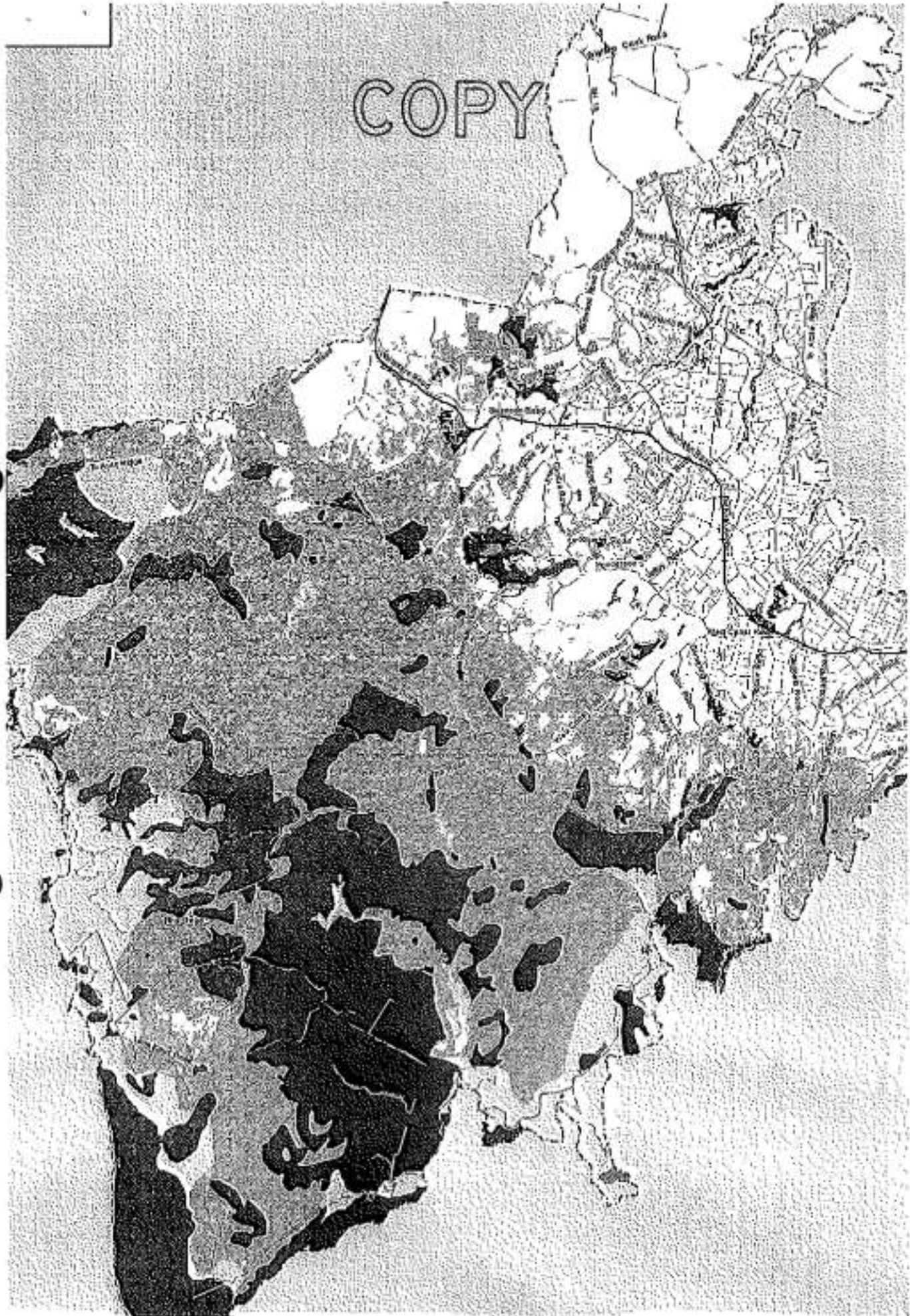
Yours faithfully  
DESIGN MANAGEMENT CONSULTANTS LTD

LYALL GREEN  
BE (Hons) CPEng IntPE # 46696

M:\2011\8008\8008 LT AC 02.DOCX

COPY

Swing Gate Falls





This website has changed

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### District Plan Natural Area Maps

Note: You will need to have [Adobe Acrobat Reader](#) installed on your computer in order to view and print these documents. For help opening PDF files or tips on copying information see [Helpful Tips](#).

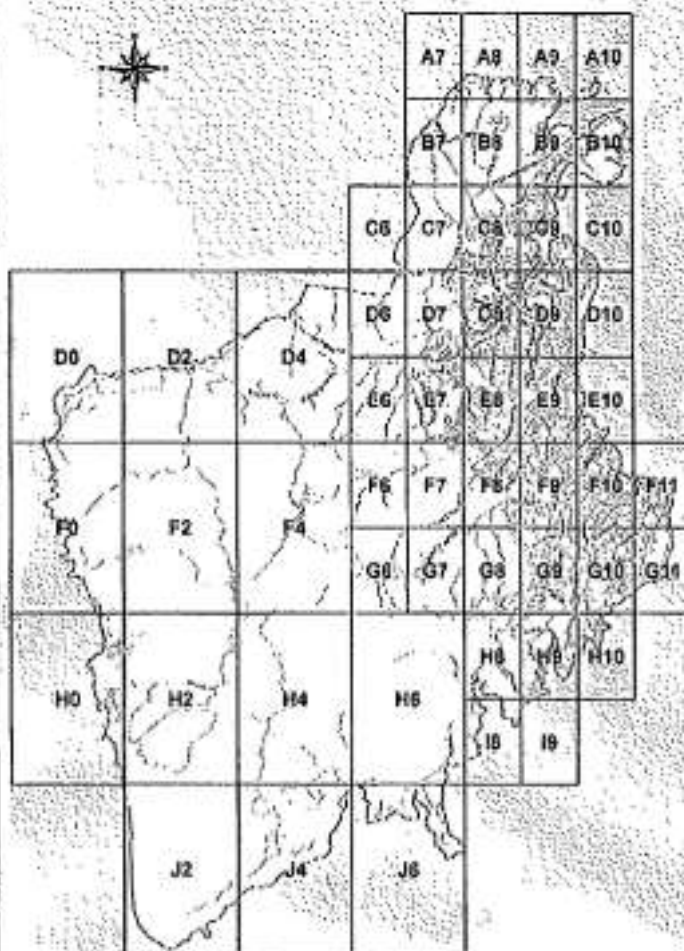
[Natural Area Map \(Size 89K\)](#)

[Road Name Index \(Size 60K\)](#)

Locate the grid reference by using this alphabetical road list.

Click the appropriate grid reference area to view the Natural Area map.

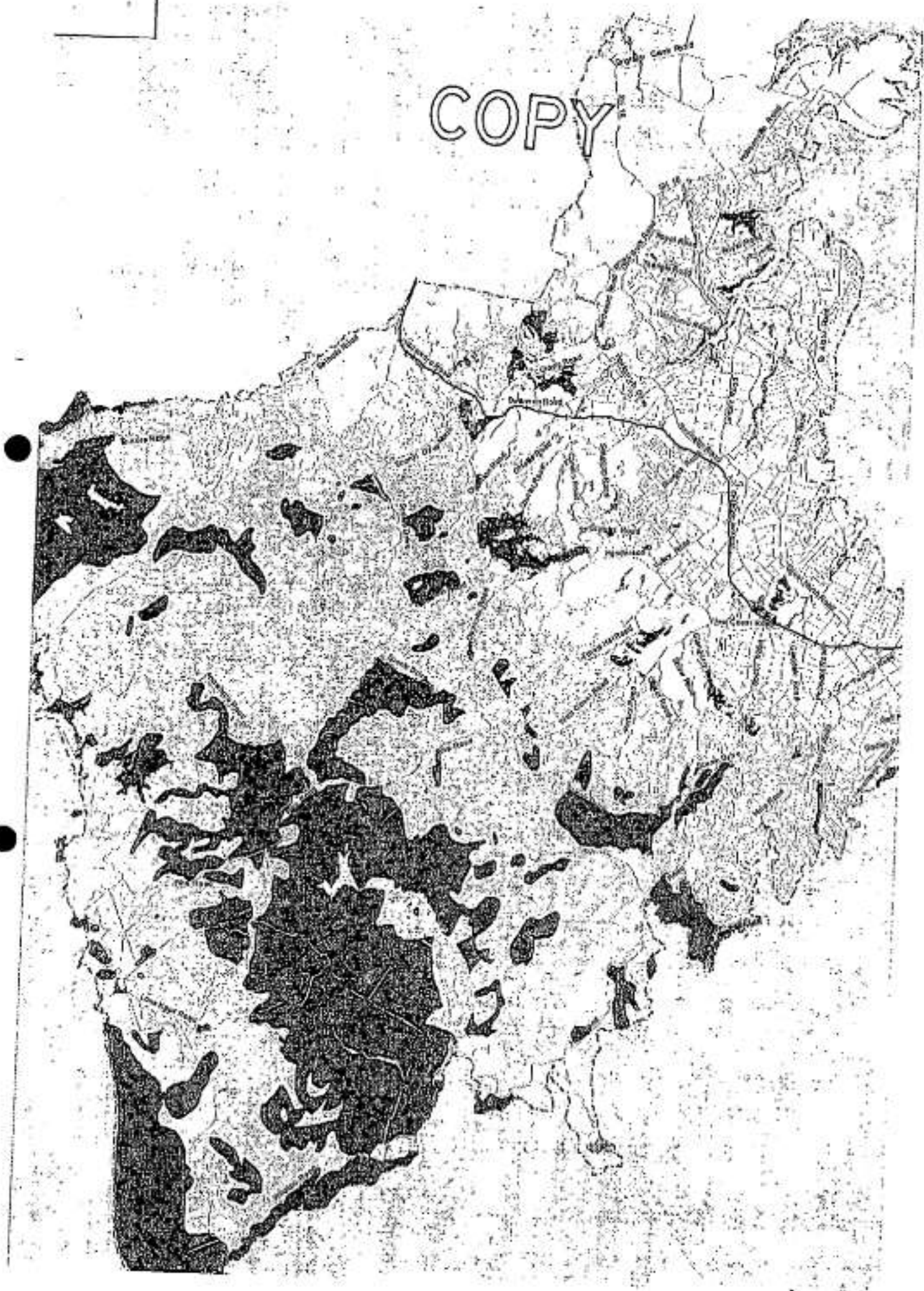
[Legend for Natural Area Maps \(Size 29K\)](#)



Page 1

[Back to District Plan Index](#)

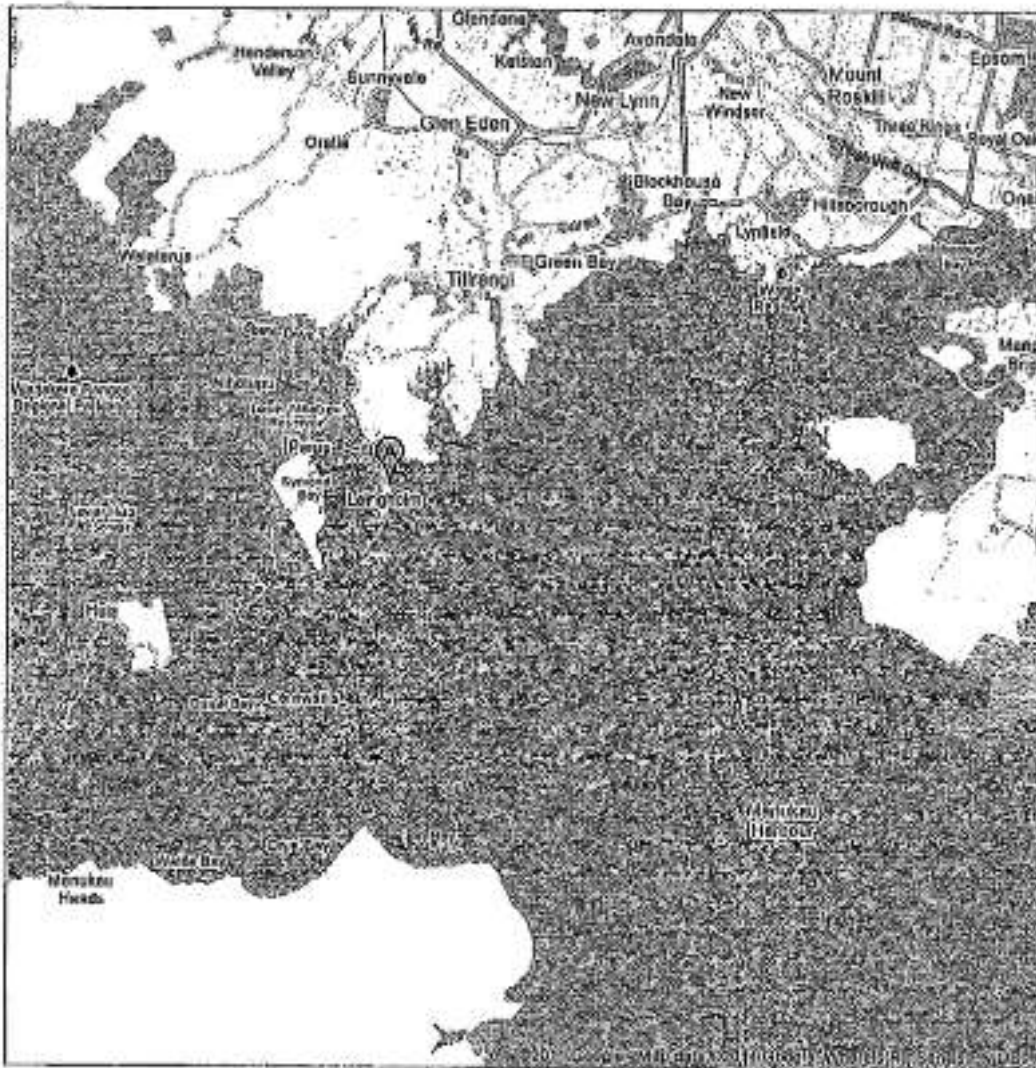
COPY





Google maps  
New Zealand

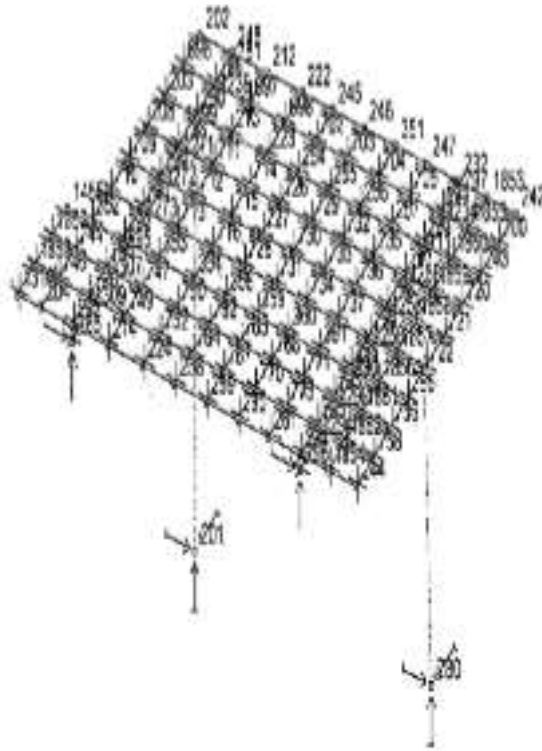
Address 52 Kauri Point Rd  
Laingholm 0604



# Microstran V9

Lyal.G  
Job: 9006 2015 Catherine's House bedroom 2 floor beams - Version 2  
B & C Moran Jones  
Floor beams

14/04/15  
07:28:42 p.



Inset: 300 phi 30

B&C Moran Jones Bed 2 floor framing



# Microstran V9

Layer 0  
 Job: 0000 2015 Cathedral House bedrom 2 floor beams - Version 2  
 B & C McFar-Jones  
 Floorbeams

## INPUT/MATERIAL REPORT

Job: 0000 2015 Cathedral House bedrom 2 floor beams - Version 2

Title: B & C McFar-Jones

2500r beam

Type: Span Beam

Date: 16 Apr 2015

Time: 08:34 AM

Nodes ..... 158  
 Members ..... 248  
 Spring supports ..... 0  
 Section ..... 8  
 Material ..... 2  
 Primary load name ..... 1  
 Distribution load cases ..... 1

Analysis: Linear elastic

## LOAD CASES

Analysis  
 Case Type Type F100 Title  
 1 F L - Dead  
 2 F L - Live  
 3 C L - W \* G  
 4 C L - 25 \* G  
 5 C L - 1.5C + 1.5G

Analysis Type:

S - Displacement analysis

L - Linear

U - Non-linear

Analysis Flag:

CRP - Censored

CRD - Designing displacements

CRN - Not used coverage in structure load

CRS - Variable on local instability

## NODE COORDINATES

Node	X	Y	Z	Material
55	3.482	0.482	2.782	000000
111	0.482	3.251	2.782	000000
167	0.482	0.399	0.999	111111
262	0.482	0.089	2.889	000000
263	0.482	0.089	2.889	000000
264	3.482	0.089	2.889	000000
265	3.482	0.089	2.889	000000
266	0.482	0.894	2.889	000000
267	0.482	0.894	2.889	000000
268	3.482	0.894	2.889	000000
269	3.482	0.894	2.889	000000
270	0.482	1.298	2.889	000000
271	0.482	1.298	2.889	000000
272	3.482	1.298	2.889	000000
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274	0.482	1.702	2.889	000000
275	0.482	1.702	2.889	000000
276	3.482	1.702	2.889	000000
277	3.482	1.702	2.889	000000
278	0.482	2.106	2.889	000000
279	0.482	2.106	2.889	000000
280	3.482	2.106	2.889	000000
281	3.482	2.106	2.889	000000
282	0.482	2.510	2.889	000000
283	0.482	2.510	2.889	000000
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389	3.482	13.014	2.889	000000
390	0.482	13.418	2.889	000000
391	0.482	13.418	2.889	000000
392	3.482	13.418	2.889	000000
393	3.482	13.418	2.889	000000
394	0.482	13.822	2.889	000000
395	0.482	13.822	2.889	000000
396	3.482	13.822	2.889	000000
397	3.482	13.822	2.889	000000
398	0.482	14.226	2.889	000000
399	0.482	14.226	2.889	000000
400	3.482	14.226	2.889	000000
401	3.482	14.226	2.889	000000
402	0.482	14.630	2.889	000000
403	0.482	14.630	2.889	000000
404	3.482	14.630	2.889	000000
405	3.482	14.630	2.889	000000
406	0.482	15.034	2.889	000000
407	0.482	15.034	2.889	000000
408	3.482	15.034	2.889	000000
409	3.482	15.034	2.889	000000
410	0.482	15.438	2.889	000000
411	0.482	15.438	2.889	000000
412	3.482	15.438	2.889	000000
413	3.482	15.438	2.889	000000
414	0.482	15.842	2.889	000000
415	0.482	15.842	2.889	000000
416	3.482	15.842	2.889	000000
417	3.482	15.842	2.889	000000
418	0.482	16.246	2.889	000000
419	0.482	16.246	2.889	000000
420	3.482	16.246	2.889	000000
421	3.482	16.246	2.889	000000
422	0.482	16.650	2.889	000000
423	0.482	16.650	2.889	000000
424	3.482	16.650	2.889	000000
425	3.482	16.650	2.889	000000
426	0.482	17.054	2.889	000000
427	0.482	17.054	2.889	000000
428	3.482	17.054	2.889	000000
429	3.482	17.054	2.889	000000
430	0.482	17.458	2.889	000000
431	0.482	17.458	2.889	000000
432	3.482	17.458	2.889	000000
433	3.482	17.458	2.889	000000
434	0.482	17.862	2.889	000000
435	0.482	17.862	2.889	000000
436	3.482	17.862	2.889	000000
437	3.482	17.862	2.889	000000
438	0.482	18.266	2.889	000000
439	0.482	18.266	2.889	000000
440	3.482	18.266	2.889	000000
441	3.482	18.266	2.889	000000
442	0.482	18.670	2.889	000000
443	0.482	18.670	2.889	000000
444	3.482	18.670	2.889	000000
445	3.482	18.670	2.889	000000
446	0.482	19.074	2.889	000000
447	0.482	19.074	2.889	000000
448	3.482	19.074	2.889	000000
449	3.482	19.074	2.889	000000
450	0.482	19.478	2.889	000000
451	0.482	19.478	2.889	000000
452	3.482	19.478	2.889	000000
453	3.482	19.478	2.889	000000
454	0.482	19.882	2.889	000000
455	0.482	19.882	2.889	000000
456	3.482	19.882	2.889	000000
457	3.482	19.882	2.889	000000
458	0.482	20.286	2.889	000000
459	0.482	20.286	2.889	000000
460	3.482	20.286	2.889	000000
461	3.482	20.286	2.889	000000

































# Microstran V9

Level: 0  
 Job: 9080 2015 Calverley House bedroom 2 floor beams - Version 2  
 G & G Micro-Gen  
 Floor beams

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 15/04/15  
 09:24:37.6

Node	X	Y	Z	U	V	W	MX	MY	MZ
1158	1005	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1159	1000	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1160	1000	-0.94	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1161	1000	-0.12	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1162	1000	-0.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1163	1000	-0.94	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1164	1005	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1165	1010	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1166	1015	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1167	1020	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1168	1025	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1169	1030	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1170	1035	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1171	1040	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1172	1045	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1173	1050	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1174	1055	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1175	1060	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1176	1065	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1177	1070	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1178	1075	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1179	1080	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1180	1085	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1181	1090	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1182	1095	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1183	1100	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1184	1105	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1185	1110	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1186	1115	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1187	1120	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1188	1125	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1189	1130	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1190	1135	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1191	1140	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1192	1145	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1193	1150	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1194	1155	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1195	1160	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1196	1165	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1197	1170	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1198	1175	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1199	1180	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1200	1185	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1201	1190	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1202	1195	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1203	1200	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1204	1205	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1205	1210	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1206	1215	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1207	1220	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1208	1225	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1209	1230	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1210	1235	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1211	1240	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1212	1245	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1213	1250	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1214	1255	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1215	1260	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1216	1265	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1217	1270	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1218	1275	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1219	1280	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1220	1285	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1221	1290	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1222	1295	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1223	1300	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1224	1305	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1225	1310	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1226	1315	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1227	1320	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1228	1325	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1229	1330	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1230	1335	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1231	1340	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1232	1345	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1233	1350	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1234	1355	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1235	1360	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1236	1365	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1237	1370	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1238	1375	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1239	1380	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1240	1385	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1241	1390	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1242	1395	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1243	1400	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1244	1405	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1245	1410	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1246	1415	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1247	1420	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1248	1425	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1249	1430	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00
1250	1435	-2.73	0.00	0.00	0.00	0.00	-8.07	0.00	0.00

Positive Faces (Order Dependent):  
 1241 - Top face  
 1242 - Right-hand side  
 1243 - End 3 support  
 1244 - Bottom  
 1245 - Slab top

### MEMBER RESULTS

Case: S1 1.20 + 1.50

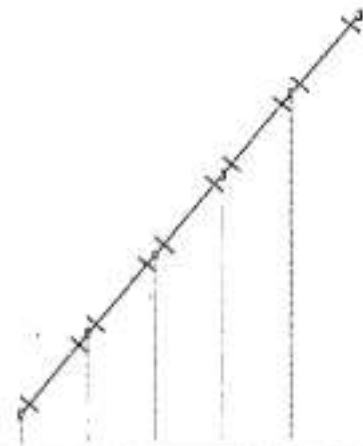
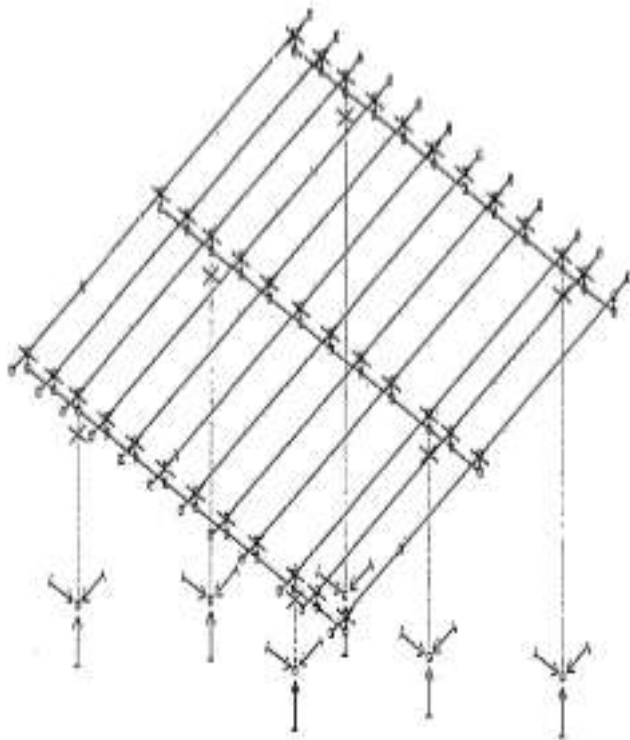
Node	Force-X	Force-Y	Force-Z	Moment-X	Moment-Y	Moment-Z
231	0.00	0.00	14.73	-0.01	0.00	0.00
793	0.00	-0.03	14.73	-0.01	0.00	0.00
795	-0.23	-0.12	0.00	0.00	0.00	0.00
796	0.23	0.03	0.00	0.00	0.00	0.00

Units: kN, kNm  
 (Reactions are in direction in positive global axis directions.)

# Microtran V9

Lynn G  
Job: 6006 2015 Catherine's House bedroom 3 floor beams  
B & C Morton-Jones  
Floor beams

14/04/15  
07:47:27 p.



theta: 310 phi: 30

BSC Morton Jones Bed 3 floor framing



# Microstran V9

Lyell,G

Job: 9006 2015 Catherine's House bedroom 3 floor beams

B & C Martin-Jones

Floor beams

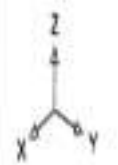
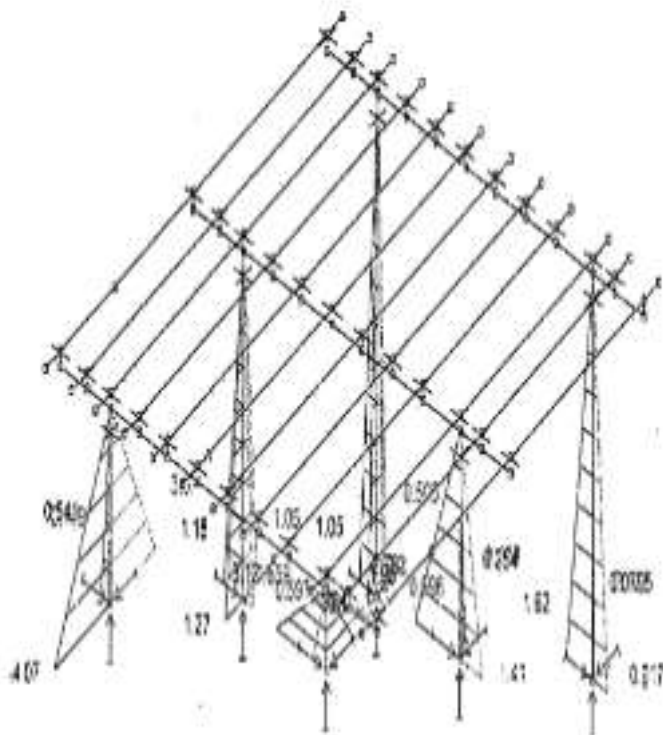
14/04/15

07:48:22 p

Load Cases:

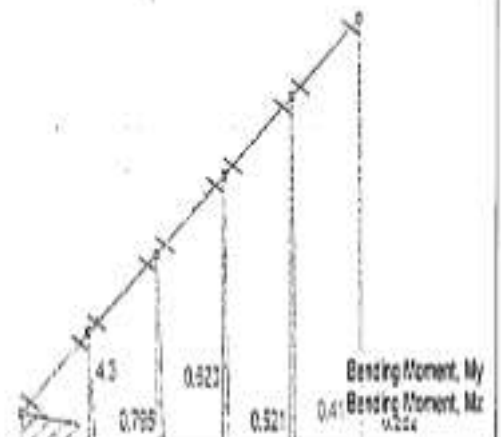
— 6 P lateral load X

— 7 P lateral load Y



theta: 310 phi: 30

B&C Martin Jones Bed 3 floor beams.rng



# Microstran V9

Level 0  
 Job: 2005-2015 Catherine House bedroom 3 floor beams  
 S & C 10046-0000  
 Floor beams

Page 1 of 14  
 16/04/15  
 08:32:43 a

## INPUT/ANALYSIS REPORT

Job: 2005-2015 Catherine House bedroom 3 floor beams

Title: S & C 10046-0000

Floor beams

Type: Spec Elems

Date: 25 Apr 2015

Time: 08:32 AM

Nodes ..... 158  
 Members ..... 134  
 Spring supports ..... 0  
 Section ..... 11  
 Materials ..... 2  
 Primary Data cases ..... 2  
 Combination load cases ..... 3

Analysis: Linear elastic

## LOAD CASES

Case	Type	Analysis	Flag	Title
1	F	L		Dead
2	F	L		Live
3	F	L		Lateral load X
4	F	L		Lateral load Y
5	C	L		G + Q
6	C	L		25 + G
7	C	L		1.25 + 1.50

Analysis Types:

1 - Staged (not analysed)

2 - Linear

3 - Non-linear

Analysis Flag:

CEX - Converge

SD2 - Excessive displacements

SD3 - End not covered in location limit

SD4 - Instable or local instability

## NODE COORDINATES

Node	X	Y	Z	Material
1	4.450	6.300	2.700	000000
2	4.450	6.900	2.700	000000
3	0.900	6.900	2.700	000000
4	0.250	6.900	2.700	000000
5	0.250	6.300	2.700	000000
6	2.450	6.300	2.700	000000
7	2.450	6.900	2.700	000000
8	4.450	6.900	2.700	000000
9	0.250	4.347	2.600	000000
10	0.250	4.347	2.700	000000
11	0.250	4.347	2.800	000000
12	2.450	4.347	2.700	000000
13	2.450	4.347	2.800	000000
14	4.450	4.347	2.800	000000
15	4.450	4.347	2.700	000000
16	4.450	4.351	2.800	000000
17	4.450	6.700	1.900	111111
18	4.450	6.700	2.700	000000
19	4.450	6.700	2.800	000000
20	0.250	6.700	-1.400	111111
21	1.450	6.700	0.200	000000
22	0.250	6.700	2.800	000000
23	0.250	6.700	2.700	000000
24	0.250	6.700	2.600	000000
25	2.450	6.700	2.700	000000
26	2.450	6.700	2.800	000000
27	4.450	6.700	2.800	000000
28	4.450	6.700	2.700	000000
29	4.450	6.700	2.600	000000
30	0.900	2.100	2.800	000000
31	0.250	2.100	2.700	000000
32	0.250	2.100	2.800	000000
33	2.450	2.100	2.700	000000
34	2.450	2.100	2.800	000000
35	4.450	2.100	2.800	000000
36	4.450	2.100	2.700	000000
37	4.450	2.100	2.600	000000
38	0.900	1.500	2.800	000000
39	0.250	1.500	2.700	000000
40	0.250	1.500	2.800	000000
41	2.450	1.500	2.700	000000
42	2.450	1.500	2.800	000000
43	4.450	1.500	2.800	000000
44	4.450	1.500	2.700	000000
45	4.450	1.500	2.600	000000
46	0.900	1.200	2.800	000000
47	0.250	1.200	2.700	000000
48	0.250	1.200	2.800	000000
49	2.450	1.200	2.700	000000
50	2.450	1.200	2.800	000000
51	4.450	1.200	2.800	000000
52	4.450	1.200	2.700	000000
53	4.450	1.200	2.600	000000
54	1.900	2.340	2.800	000000
55	0.900	2.340	2.800	000000
56	0.250	2.340	2.700	000000
57	0.250	2.340	2.800	000000
58	2.450	2.340	2.700	000000
59	2.450	2.340	2.800	000000
60	4.450	2.340	2.800	000000
61	4.450	2.340	2.700	000000
62	4.450	2.340	2.600	000000
63	0.900	2.700	2.800	000000
64	0.250	2.700	2.700	000000
65	0.250	2.700	2.800	000000
66	2.450	2.700	2.700	000000
67	2.450	2.700	2.800	000000
68	4.450	2.700	2.800	000000
69	4.450	2.700	2.700	000000
70	4.450	2.700	2.600	000000
71	0.250	3.100	2.800	000000
72	0.250	3.100	2.700	000000
73	2.450	3.100	2.800	000000
74	2.450	3.100	2.700	000000
75	4.450	3.100	2.800	000000
76	4.450	3.100	2.700	000000
77	4.450	3.100	2.600	000000
78	0.900	3.700	2.800	111111
79	0.250	3.700	2.700	000000
80	0.250	3.700	2.800	000000
81	2.450	3.700	2.700	000000
82	2.450	3.700	2.800	000000
83	4.450	3.700	2.800	111111
84	4.450	3.700	2.700	000000
85	4.450	3.700	2.600	000000
86	0.900	4.300	2.800	000000
87	0.250	4.300	2.700	000000
88	0.250	4.300	2.800	000000

89	0.250	4.300	2.700	000000
90	2.450	4.300	2.700	000000
91	2.450	4.300	2.800	000000
92	4.450	4.300	2.700	000000
93	4.450	4.300	2.800	000000
94	4.450	4.300	2.600	000000
95	4.450	4.300	2.700	000000
96	4.450	4.300	2.800	000000
97	0.250	4.900	2.800	000000
98	0.250	4.900	2.700	000000
99	0.250	4.900	2.600	000000
100	2.450	4.900	2.700	000000
101	2.450	4.900	2.800	000000
102	4.450	4.900	2.800	000000
103	4.450	4.900	2.700	000000
104	4.450	4.900	2.600	000000
105	4.450	4.900	2.700	111111
106	4.450	4.900	2.800	000000
107	0.250	10.700	-0.400	111111
108	2.450	10.700	0.500	111111
109	2.450	10.700	2.700	000000
110	2.450	10.700	2.800	000000
111	4.450	10.700	2.700	000000
112	4.450	10.700	2.800	000000
113	4.450	10.700	2.600	000000
114	4.450	10.700	2.700	000000
115	4.450	10.700	2.800	111111
116	1.250	10.700	0.200	111111
117	1.250	10.700	2.700	000000
118	1.250	10.700	2.800	111111
119	2.450	10.700	1.900	000000

## WATER/SLAB CONSTRAINTS

Node	X	Y	Z	Rel-A	Rel-B	Rel-C	Rel-D
1	50	50	0	0	0	0	0
2	50	50	0	0	0	0	0
3	50	50	0	0	0	0	0
4	50	50	0	0	0	0	0
5	50	50	0	0	0	0	0
6	50	50	0	0	0	0	0
7	50	50	0	0	0	0	0
8	50	50	0	0	0	0	0
9	50	50	0	0	0	0	0
10	50	50	0	0	0	0	0
11	50	50	0	0	0	0	0
12	50	50	0	0	0	0	0
13	50	50	0	0	0	0	0
14	50	50	0	0	0	0	0
15	50	50	0	0	0	0	0
16	50	50	0	0	0	0	0
17	50	50	0	0	0	0	0
18	50	50	0	0	0	0	0
19	50	50	0	0	0	0	0
20	50	50	0	0	0	0	0
21	50	50	0	0	0	0	0
22	50	50	0	0	0	0	0
23	50	50	0	0	0	0	0
24	50	50	0	0	0	0	0
25	50	50	0	0	0	0	0
26	50	50	0	0	0	0	0
27	50	50	0	0	0	0	0
28	50	50	0	0	0	0	0
29	50	50	0	0	0	0	0
30	50	50	0	0	0	0	0
31	50	50	0	0	0	0	0
32	50	50	0	0	0	0	0
33	50	50	0	0	0	0	0
34	50	50	0	0	0	0	0
35	50	50	0	0	0	0	0
36	50	50	0	0	0	0	0
37	50	50	0	0	0	0	0
38	50	50	0	0	0	0	0
39	50	50	0	0	0	0	0
40	50	50	0	0	0	0	0
41	50	50	0	0	0	0	0
42	50	50	0	0	0	0	0
43	50	50	0	0	0	0	0
44	50	50	0	0	0	0	0
45	50	50	0	0	0	0	0
46	50	50	0	0	0	0	0
47	50	50	0	0	0	0	0
48	50	50	0	0	0	0	0
49	50	50	0	0	0	0	0
50	50	50	0	0	0	0	0
51	50	50	0	0	0	0	0
52	50	50	0	0	0	0	0
53	50	50	0	0	0	0	0
54	50	50	0	0	0	0	0
55	50	50	0	0	0	0	0
56	50	50	0	0	0	0	0
57	50	50	0	0	0	0	0
58	50	50	0	0	0	0	0
59	50	50	0	0	0	0	0
60	50	50	0	0	0	0	0
61	50	50	0	0	0	0	0
62	50	50	0	0	0	0	0
63	50	50	0	0	0	0	0
64	50	50	0	0	0	0	0
65	50	50	0	0	0	0	0
66	50	50	0	0	0	0	0
67	50	50	0	0	0	0	0
68	50	50	0	0	0	0	0
69	50	50	0	0	0	0	0
70	50	50	0	0	0	0	0
71	50	50	0	0	0	0	0
72	50	50	0	0	0	0	0
73	50	50	0	0	0	0	0
74	50	50	0	0	0	0	0
75	50	50	0	0	0	0	0
76	50	50	0	0	0	0	0
77</							

# Microstran V9

Light  
Job: 0008 2015 Colchester House between 2 floor beams  
B & G (Main) (1) 1  
Floor beam

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15/04/15  
08:31:43 a

Node	X	Y	Z	U	V	W	U	V	W
42	47	56	1	88	1.000000	0.000000	0.420		
43	48	57	1	88	1.000000	0.000000	0.420		
44	49	58	-X	88	1.000000	0.000000	0.420		
45	50	59	1	88	1.000000	0.000000	0.420		
46	51	60	1	88	1.000000	0.000000	0.420		
47	52	61	-X	88	1.000000	0.000000	0.420		
48	53	62	1	88	1.000000	0.000000	0.420		
49	54	63	-X	88	1.000000	0.000000	0.420		
50	55	64	1	88	1.000000	0.000000	0.420		
51	56	65	1	88	1.000000	0.000000	0.420		
52	57	66	1	88	1.000000	0.000000	0.420		
53	58	67	-X	88	1.000000	0.000000	0.420		
54	59	68	1	88	1.000000	0.000000	0.420		
55	60	69	1	88	1.000000	0.000000	0.420		
56	61	70	-X	88	1.000000	0.000000	0.420		
57	62	71	1	88	1.000000	0.000000	0.420		
58	63	72	1	88	1.000000	0.000000	0.420		
59	64	73	-X	88	1.000000	0.000000	0.420		
60	65	74	1	88	1.000000	0.000000	0.420		
61	66	75	1	88	1.000000	0.000000	0.420		
62	67	76	-X	88	1.000000	0.000000	0.420		
63	68	77	1	88	1.000000	0.000000	0.420		
64	69	78	1	88	1.000000	0.000000	0.420		
65	70	79	-X	88	1.000000	0.000000	0.420		
66	71	80	1	88	1.000000	0.000000	0.420		
67	72	81	1	88	1.000000	0.000000	0.420		
68	73	82	-X	88	1.000000	0.000000	0.420		
69	74	83	1	88	1.000000	0.000000	0.420		
70	75	84	1	88	1.000000	0.000000	0.420		
71	76	85	-X	88	1.000000	0.000000	0.420		
72	77	86	1	88	1.000000	0.000000	0.420		
73	78	87	1	88	1.000000	0.000000	0.420		
74	79	88	-X	88	1.000000	0.000000	0.420		
75	80	89	1	88	1.000000	0.000000	0.420		
76	81	90	1	88	1.000000	0.000000	0.420		
77	82	91	-X	88	1.000000	0.000000	0.420		
78	83	92	1	88	1.000000	0.000000	0.420		
79	84	93	1	88	1.000000	0.000000	0.420		
80	85	94	-X	88	1.000000	0.000000	0.420		
81	86	95	1	88	1.000000	0.000000	0.420		
82	87	96	1	88	1.000000	0.000000	0.420		
83	88	97	-X	88	1.000000	0.000000	0.420		
84	89	98	1	88	1.000000	0.000000	0.420		
85	90	99	1	88	1.000000	0.000000	0.420		
86	91	100	-X	88	1.000000	0.000000	0.420		
87	92	101	1	88	1.000000	0.000000	0.420		
88	93	102	1	88	1.000000	0.000000	0.420		
89	94	103	-X	88	1.000000	0.000000	0.420		
90	95	104	1	88	1.000000	0.000000	0.420		
91	96	105	1	88	1.000000	0.000000	0.420		
92	97	106	-X	88	1.000000	0.000000	0.420		
93	98	107	1	88	1.000000	0.000000	0.420		
94	99	108	1	88	1.000000	0.000000	0.420		
95	100	109	-X	88	1.000000	0.000000	0.420		
96	101	110	1	88	1.000000	0.000000	0.420		
97	102	111	1	88	1.000000	0.000000	0.420		
98	103	112	-X	88	1.000000	0.000000	0.420		
99	104	113	1	88	1.000000	0.000000	0.420		
100	105	114	1	88	1.000000	0.000000	0.420		
101	106	115	-X	88	1.000000	0.000000	0.420		
102	107	116	1	88	1.000000	0.000000	0.420		
103	108	117	1	88	1.000000	0.000000	0.420		
104	109	118	-X	88	1.000000	0.000000	0.420		
105	110	119	1	88	1.000000	0.000000	0.420		
106	111	120	1	88	1.000000	0.000000	0.420		
107	112	121	-X	88	1.000000	0.000000	0.420		
108	113	122	1	88	1.000000	0.000000	0.420		
109	114	123	1	88	1.000000	0.000000	0.420		
110	115	124	-X	88	1.000000	0.000000	0.420		
111	116	125	1	88	1.000000	0.000000	0.420		
112	117	126	1	88	1.000000	0.000000	0.420		
113	118	127	-X	88	1.000000	0.000000	0.420		
114	119	128	1	88	1.000000	0.000000	0.420		
115	120	129	1	88	1.000000	0.000000	0.420		
116	121	130	-X	88	1.000000	0.000000	0.420		
117	122	131	1	88	1.000000	0.000000	0.420		
118	123	132	1	88	1.000000	0.000000	0.420		
119	124	133	-X	88	1.000000	0.000000	0.420		
120	125	134	1	88	1.000000	0.000000	0.420		
121	126	135	1	88	1.000000	0.000000	0.420		
122	127	136	-X	88	1.000000	0.000000	0.420		
123	128	137	1	88	1.000000	0.000000	0.420		
124	129	138	1	88	1.000000	0.000000	0.420		
125	130	139	-X	88	1.000000	0.000000	0.420		
126	131	140	1	88	1.000000	0.000000	0.420		
127	132	141	1	88	1.000000	0.000000	0.420		
128	133	142	-X	88	1.000000	0.000000	0.420		
129	134	143	1	88	1.000000	0.000000	0.420		
130	135	144	1	88	1.000000	0.000000	0.420		
131	136	145	-X	88	1.000000	0.000000	0.420		
132	137	146	1	88	1.000000	0.000000	0.420		
133	138	147	1	88	1.000000	0.000000	0.420		
134	139	148	-X	88	1.000000	0.000000	0.420		
135	140	149	1	88	1.000000	0.000000	0.420		
136	141	150	1	88	1.000000	0.000000	0.420		
137	142	151	-X	88	1.000000	0.000000	0.420		
138	143	152	1	88	1.000000	0.000000	0.420		
139	144	153	1	88	1.000000	0.000000	0.420		
140	145	154	-X	88	1.000000	0.000000	0.420		
141	146	155	1	88	1.000000	0.000000	0.420		
142	147	156	1	88	1.000000	0.000000	0.420		
143	148	157	-X	88	1.000000	0.000000	0.420		
144	149	158	1	88	1.000000	0.000000	0.420		
145	150	159	1	88	1.000000	0.000000	0.420		
146	151	160	-X	88	1.000000	0.000000	0.420		
147	152	161	1	88	1.000000	0.000000	0.420		
148	153	162	1	88	1.000000	0.000000	0.420		
149	154	163	-X	88	1.000000	0.000000	0.420		
150	155	164	1	88	1.000000	0.000000	0.420		
151	156	165	1	88	1.000000	0.000000	0.420		
152	157	166	-X	88	1.000000	0.000000	0.420		
153	158	167	1	88	1.000000	0.000000	0.420		
154	159	168	1	88	1.000000	0.000000	0.420		
155	160	169	-X	88	1.000000	0.000000	0.420		
156	161	170	1	88	1.000000	0.000000	0.420		
157	162	171	1	88	1.000000	0.000000	0.420		
158	163	172	-X	88	1.000000	0.000000	0.420		
159	164	173	1	88	1.000000	0.000000	0.420		
160	165	174	1	88	1.000000	0.000000	0.420		
161	166	175	-X	88	1.000000	0.000000	0.420		
162	167	176	1	88	1.000000	0.000000	0.420		
163	168	177	1	88	1.000000	0.000000	0.420		
164	169	178	-X	88	1.000000	0.000000	0.420		
165	170	179	1	88	1.000000	0.000000	0.420		
166	171	180	1	88	1.000000	0.000000	0.420		
167	172	181	-X	88	1.000000	0.000000	0.420		
168	173	182	1	88	1.000000	0.000000	0.420		
169	174	183	1	88	1.000000	0.000000	0.420		
170	175	184	-X	88	1.000000	0.000000	0.420		
171	176	185	1	88	1.000000	0.000000	0.420		
172	177	186	1	88	1.000000	0.000000	0.420		
173	178	187	-X	88	1.000000	0.000000	0.420		
174	179	188	1	88	1.000000	0.000000	0.420		
175	180	189	1	88	1.000000	0.000000	0.420		
176	181	190	-X	88	1.000000	0.000000	0.420		
177	182	191	1	88	1.000000	0.000000	0.420		
178	183	192	1	88	1.000000	0.000000	0.420		
179	184	193	-X	88	1.000000	0.000000	0.420		
180	185	194	1	88	1.000000	0.000000	0.420		
181	186	195	1	88	1.000000	0.000000	0.420		
182	187	196	-X	88	1.000000	0.000000	0.420		
183	188	197	1	88	1.000000	0.000000	0.420		
184	189	198</							





# Microstran V9

Light  
Job: 0608 2015 Cathedral House bedrooms 3 floor beams  
B & C Marked Areas  
Floor beams

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16/06/15  
08:33:43

Node	X Force	Y Force	Z Force	X Moment	Y Moment	Z Moment	
87	12	8.93	0.73	0.68	8.00	0.33	-0.18
88	12	-0.32	8.90	0.69	8.00	0.33	8.00
89	12	-0.32	2.00	0.68	8.00	0.33	0.90
90	12	0.93	-0.10	0.69	8.00	0.33	-0.18
91	12	0.93	8.10	0.69	8.00	0.33	-0.18
92	12	-0.22	8.00	0.68	8.00	0.33	0.90
93	12	-0.22	0.00	0.68	8.00	0.33	0.90
94	12	0.93	-0.12	0.69	8.00	0.33	-0.18
95	12	0.93	8.17	0.69	8.00	0.33	-0.18
96	12	0.00	2.09	0.69	8.00	0.33	-0.18
97	12	0.00	2.58	0.69	8.00	0.33	-0.18
98	12	0.00	2.58	0.69	8.00	0.33	-0.18
99	12	0.00	0.00	0.69	8.00	0.33	-0.18
100	12	0.00	0.00	0.69	8.00	0.33	-0.18
101	12	0.00	0.00	0.69	8.00	0.33	-0.18
102	12	0.00	0.00	0.69	8.00	0.33	-0.18
103	12	0.00	0.00	0.69	8.00	0.33	-0.18
104	12	0.00	0.00	0.69	8.00	0.33	-0.18
105	12	0.00	0.00	0.69	8.00	0.33	-0.18
106	12	0.00	0.00	0.69	8.00	0.33	-0.18
107	12	0.00	0.00	0.69	8.00	0.33	-0.18
108	12	0.00	0.00	0.69	8.00	0.33	-0.18
109	12	0.00	0.00	0.69	8.00	0.33	-0.18
110	12	0.00	0.00	0.69	8.00	0.33	-0.18
111	12	0.00	0.00	0.69	8.00	0.33	-0.18
112	12	0.00	0.00	0.69	8.00	0.33	-0.18
113	12	0.00	0.00	0.69	8.00	0.33	-0.18
114	12	0.00	0.00	0.69	8.00	0.33	-0.18
115	12	0.00	0.00	0.69	8.00	0.33	-0.18
116	12	0.00	0.00	0.69	8.00	0.33	-0.18
117	12	0.00	0.00	0.69	8.00	0.33	-0.18
118	12	0.00	0.00	0.69	8.00	0.33	-0.18
119	12	0.00	0.00	0.69	8.00	0.33	-0.18
120	12	0.00	0.00	0.69	8.00	0.33	-0.18
121	12	0.00	0.00	0.69	8.00	0.33	-0.18
122	12	0.00	0.00	0.69	8.00	0.33	-0.18
123	12	0.00	0.00	0.69	8.00	0.33	-0.18
124	12	0.00	0.00	0.69	8.00	0.33	-0.18
125	12	0.00	0.00	0.69	8.00	0.33	-0.18
126	12	0.00	0.00	0.69	8.00	0.33	-0.18
127	12	0.00	0.00	0.69	8.00	0.33	-0.18
128	12	0.00	0.00	0.69	8.00	0.33	-0.18
129	12	0.00	0.00	0.69	8.00	0.33	-0.18
130	12	0.00	0.00	0.69	8.00	0.33	-0.18
131	12	0.00	0.00	0.69	8.00	0.33	-0.18
132	12	0.00	0.00	0.69	8.00	0.33	-0.18
133	12	0.00	0.00	0.69	8.00	0.33	-0.18
134	12	0.00	0.00	0.69	8.00	0.33	-0.18
135	12	0.00	0.00	0.69	8.00	0.33	-0.18
136	12	0.00	0.00	0.69	8.00	0.33	-0.18
137	12	0.00	0.00	0.69	8.00	0.33	-0.18
138	12	0.00	0.00	0.69	8.00	0.33	-0.18
139	12	0.00	0.00	0.69	8.00	0.33	-0.18
140	12	0.00	0.00	0.69	8.00	0.33	-0.18
141	12	0.00	0.00	0.69	8.00	0.33	-0.18
142	12	0.00	0.00	0.69	8.00	0.33	-0.18
143	12	0.00	0.00	0.69	8.00	0.33	-0.18
144	12	0.00	0.00	0.69	8.00	0.33	-0.18
145	12	0.00	0.00	0.69	8.00	0.33	-0.18
146	12	0.00	0.00	0.69	8.00	0.33	-0.18
147	12	0.00	0.00	0.69	8.00	0.33	-0.18
148	12	0.00	0.00	0.69	8.00	0.33	-0.18
149	12	0.00	0.00	0.69	8.00	0.33	-0.18
150	12	0.00	0.00	0.69	8.00	0.33	-0.18
151	12	0.00	0.00	0.69	8.00	0.33	-0.18
152	12	0.00	0.00	0.69	8.00	0.33	-0.18
153	12	0.00	0.00	0.69	8.00	0.33	-0.18
154	12	0.00	0.00	0.69	8.00	0.33	-0.18
155	12	0.00	0.00	0.69	8.00	0.33	-0.18
156	12	0.00	0.00	0.69	8.00	0.33	-0.18
157	12	0.00	0.00	0.69	8.00	0.33	-0.18
158	12	0.00	0.00	0.69	8.00	0.33	-0.18
159	12	0.00	0.00	0.69	8.00	0.33	-0.18
160	12	0.00	0.00	0.69	8.00	0.33	-0.18
161	12	0.00	0.00	0.69	8.00	0.33	-0.18
162	12	0.00	0.00	0.69	8.00	0.33	-0.18
163	12	0.00	0.00	0.69	8.00	0.33	-0.18
164	12	0.00	0.00	0.69	8.00	0.33	-0.18

Positive Forces Member Area:  
Axial - Tension  
Torsion - Right-hand twist

Shear - End A sagging  
Moment - Sagging

### WASHER REACTIONS

Node	Y-Disp	Z-Disp	Y-Rot	Z-Rot
17	0.00	0.00	0.00	0.00
20	0.00	0.00	0.00	0.00
21	0.00	0.00	0.00	0.00
45	0.00	0.00	0.00	0.00
79	0.00	0.00	0.00	0.00
80	0.00	0.00	0.00	0.00
83	0.00	0.00	0.00	0.00
130	0.00	0.00	0.00	0.00
139	0.00	0.00	0.00	0.00
144	0.00	0.00	0.00	0.00
149	0.00	0.00	0.00	0.00
150	0.00	0.00	0.00	0.00
151	0.00	0.00	0.00	0.00
152	0.00	0.00	0.00	0.00
153	0.00	0.00	0.00	0.00
154	0.00	0.00	0.00	0.00
155	0.00	0.00	0.00	0.00
156	0.00	0.00	0.00	0.00
157	0.00	0.00	0.00	0.00
158	0.00	0.00	0.00	0.00
159	0.00	0.00	0.00	0.00
160	0.00	0.00	0.00	0.00
161	0.00	0.00	0.00	0.00
162	0.00	0.00	0.00	0.00
163	0.00	0.00	0.00	0.00
164	0.00	0.00	0.00	0.00

Max. axial: -1.17E+02 at node 467

Checklist: see structure in positive global axis direction 3

### Case 2: Live

### Node Loads

Node	X Force	Y Force	Z Force	X Moment	Y Moment	Z Moment
8	0.000	0.000	-0.488	0.000	0.000	0.000
100	0.000	0.000	-0.488	0.000	0.000	0.000
103	0.000	0.000	-0.488	0.000	0.000	0.000
104	0.000	0.000	-0.488	0.000	0.000	0.000

### Member Loads

Member	Field	T	A	Z	Y1	X1	Y2	X2
2	0.000	0.000	0.000	0.000	-0.340			
3	0.000	0.000	0.000	0.000	-0.330			
4	0.000	0.000	0.000	0.000	-0.300			
11	0.000	0.000	0.000	0.000	-0.400			
12	0.000	0.000	0.000	0.000	-0.400			
13	0.000	0.000	0.000	0.000	-0.400			
14	0.000	0.000	0.000	0.000	-0.400			
15	0.000	0.000	0.000	0.000	-0.400			
16	0.000	0.000	0.000	0.000	-0.400			
17	0.000	0.000	0.000	0.000	-0.400			
18	0.000	0.000	0.000	0.000	-0.400			
19	0.000	0.000	0.000	0.000	-0.400			
20	0.000	0.000	0.000	0.000	-0.400			
21	0.000	0.000	0.000	0.000	-0.400			
22	0.000	0.000	0.000	0.000	-0.400			
23	0.000	0.000	0.000	0.000	-0.400			
24	0.000	0.000	0.000	0.000	-0.400			
25	0.000	0.000	0.000	0.000	-0.400			
26	0.000	0.000	0.000	0.000	-0.400			
27	0.000	0.000	0.000	0.000	-0.400			
28	0.000	0.000	0.000	0.000	-0.400			
29	0.000	0.000	0.000	0.000	-0.400			
30	0.000	0.000	0.000	0.000	-0.400			
31	0.000	0.000	0.000	0.000	-0.400			
32	0.000	0.000	0.000	0.000	-0.400			
33	0.000	0.000	0.000	0.000	-0.400			
34	0.000	0.000	0.000	0.000	-0.400			
35	0.000	0.000	0.000	0.000	-0.400			
36	0.000	0.000	0.000	0.000	-0.400			
37	0.000	0.000	0.000	0.000	-0.400			
38	0.000	0.000	0.000	0.000	-0.400			
39	0.000	0.000	0.000	0.000	-0.400			
40	0.000	0.000	0.000	0.000	-0.400			
41	0.000	0.000	0.000	0.000	-0.400			
42	0.000	0.000	0.000	0.000	-0.400			
43	0.000	0.000	0.000	0.000	-0.400			
44	0.000	0.000	0.000	0.000	-0.400			
45	0.000	0.000	0.000	0.000	-0.400			
46	0.000	0.000	0.000	0.000	-0.400			
47	0.000	0.000	0.000	0.000	-0.400			
48	0.000	0.000	0.000	0.000	-0.400			
49	0.000	0.000	0.000	0.000	-0.400			
50	0.000	0.000	0.000	0.000	-0.400			
51	0.000	0.000	0.000	0.000	-0.400			
52	0.000	0.000	0.000	0.000	-0.400			
53	0.000	0.000	0.000	0.000	-0.400			
54	0.000	0.000	0.000	0.000	-0.400			
55	0.000	0.000	0.000	0.000	-0.400			
56	0.000	0.000	0.000	0.000	-0.400			
57	0.000	0.000	0.000	0.000	-0.400			
58	0.000	0.000	0.000	0.000	-0.400			
59	0.000	0.000	0.000	0.000	-0.400			
60	0.000	0.000	0.000	0.000	-0.400			

Sum of Applied Loads (Global Axes):  
FX: 0.000 FY: 0.000 FZ: -20.788  
Mx: -11.943 My: 0.000 Mz: 0.000

# Microstran V9

Lyell O  
 Job: 9000 2015 Cathlamet House beam3 3 floor beams  
 B & C Median-Jones  
 Floor beams

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 09:33:43 a

82	0.0000	0.0000	-2.0000	1.4875+08	0.0000	-0.0000	0.0000
83	0.0000	0.0000	-2.0000	1.4875+08	-0.0000	0.0000	0.0000
84	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	-0.0000	0.0000
85	0.0000	0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
86	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
87	0.0000	0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
88	0.0000	0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
89	0.0000	0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
90	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
91	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
92	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
93	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
94	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
95	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
96	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
97	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
98	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
99	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
100	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
101	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
102	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
103	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
104	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
105	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
106	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
107	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
108	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
109	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
110	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
111	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
112	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
113	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
114	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
115	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
116	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
117	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
118	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
119	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
120	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
121	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000
122	0.0000	-0.0000	-0.0000	1.4875+08	-0.0000	0.0000	0.0000

MEMBER PROPERTIES

MEMBER	TYPE	AREA	INERTIA	WGT	TWIST	MASS PER UNIT LENGTH	MOI
1	1	1.00	0.00	0.00	0.00	0.00	0.00
2	2	2.00	0.00	0.00	0.00	0.00	0.00
3	3	3.00	0.00	0.00	0.00	0.00	0.00
4	4	4.00	0.00	0.00	0.00	0.00	0.00
5	5	5.00	0.00	0.00	0.00	0.00	0.00
6	6	6.00	0.00	0.00	0.00	0.00	0.00
7	7	7.00	0.00	0.00	0.00	0.00	0.00
8	8	8.00	0.00	0.00	0.00	0.00	0.00
9	9	9.00	0.00	0.00	0.00	0.00	0.00
10	10	10.00	0.00	0.00	0.00	0.00	0.00
11	11	11.00	0.00	0.00	0.00	0.00	0.00
12	12	12.00	0.00	0.00	0.00	0.00	0.00
13	13	13.00	0.00	0.00	0.00	0.00	0.00
14	14	14.00	0.00	0.00	0.00	0.00	0.00
15	15	15.00	0.00	0.00	0.00	0.00	0.00
16	16	16.00	0.00	0.00	0.00	0.00	0.00
17	17	17.00	0.00	0.00	0.00	0.00	0.00
18	18	18.00	0.00	0.00	0.00	0.00	0.00
19	19	19.00	0.00	0.00	0.00	0.00	0.00
20	20	20.00	0.00	0.00	0.00	0.00	0.00
21	21	21.00	0.00	0.00	0.00	0.00	0.00
22	22	22.00	0.00	0.00	0.00	0.00	0.00
23	23	23.00	0.00	0.00	0.00	0.00	0.00
24	24	24.00	0.00	0.00	0.00	0.00	0.00
25	25	25.00	0.00	0.00	0.00	0.00	0.00
26	26	26.00	0.00	0.00	0.00	0.00	0.00
27	27	27.00	0.00	0.00	0.00	0.00	0.00
28	28	28.00	0.00	0.00	0.00	0.00	0.00
29	29	29.00	0.00	0.00	0.00	0.00	0.00
30	30	30.00	0.00	0.00	0.00	0.00	0.00
31	31	31.00	0.00	0.00	0.00	0.00	0.00
32	32	32.00	0.00	0.00	0.00	0.00	0.00
33	33	33.00	0.00	0.00	0.00	0.00	0.00
34	34	34.00	0.00	0.00	0.00	0.00	0.00
35	35	35.00	0.00	0.00	0.00	0.00	0.00
36	36	36.00	0.00	0.00	0.00	0.00	0.00
37	37	37.00	0.00	0.00	0.00	0.00	0.00

38	38	38.00	0.00	0.00	0.00	0.00	0.00
39	39	39.00	0.00	0.00	0.00	0.00	0.00
40	40	40.00	0.00	0.00	0.00	0.00	0.00
41	41	41.00	0.00	0.00	0.00	0.00	0.00
42	42	42.00	0.00	0.00	0.00	0.00	0.00
43	43	43.00	0.00	0.00	0.00	0.00	0.00
44	44	44.00	0.00	0.00	0.00	0.00	0.00
45	45	45.00	0.00	0.00	0.00	0.00	0.00
46	46	46.00	0.00	0.00	0.00	0.00	0.00
47	47	47.00	0.00	0.00	0.00	0.00	0.00
48	48	48.00	0.00	0.00	0.00	0.00	0.00
49	49	49.00	0.00	0.00	0.00	0.00	0.00
50	50	50.00	0.00	0.00	0.00	0.00	0.00
51	51	51.00	0.00	0.00	0.00	0.00	0.00
52	52	52.00	0.00	0.00	0.00	0.00	0.00
53	53	53.00	0.00	0.00	0.00	0.00	0.00
54	54	54.00	0.00	0.00	0.00	0.00	0.00
55	55	55.00	0.00	0.00	0.00	0.00	0.00
56	56	56.00	0.00	0.00	0.00	0.00	0.00
57	57	57.00	0.00	0.00	0.00	0.00	0.00
58	58	58.00	0.00	0.00	0.00	0.00	0.00
59	59	59.00	0.00	0.00	0.00	0.00	0.00
60	60	60.00	0.00	0.00	0.00	0.00	0.00
61	61	61.00	0.00	0.00	0.00	0.00	0.00
62	62	62.00	0.00	0.00	0.00	0.00	0.00
63	63	63.00	0.00	0.00	0.00	0.00	0.00
64	64	64.00	0.00	0.00	0.00	0.00	0.00
65	65	65.00	0.00	0.00	0.00	0.00	0.00
66	66	66.00	0.00	0.00	0.00	0.00	0.00
67	67	67.00	0.00	0.00	0.00	0.00	0.00
68	68	68.00	0.00	0.00	0.00	0.00	0.00
69	69	69.00	0.00	0.00	0.00	0.00	0.00
70	70	70.00	0.00	0.00	0.00	0.00	0.00
71	71	71.00	0.00	0.00	0.00	0.00	0.00
72	72	72.00	0.00	0.00	0.00	0.00	0.00
73	73	73.00	0.00	0.00	0.00	0.00	0.00
74	74	74.00	0.00	0.00	0.00	0.00	0.00
75	75	75.00	0.00	0.00	0.00	0.00	0.00
76	76	76.00	0.00	0.00	0.00	0.00	0.00
77	77	77.00	0.00	0.00	0.00	0.00	0.00
78	78	78.00	0.00	0.00	0.00	0.00	0.00
79	79	79.00	0.00	0.00	0.00	0.00	0.00
80	80	80.00	0.00	0.00	0.00	0.00	0.00
81	81	81.00	0.00	0.00	0.00	0.00	0.00
82	82	82.00	0.00	0.00	0.00	0.00	0.00
83	83	83.00	0.00	0.00	0.00	0.00	0.00
84	84	84.00	0.00	0.00	0.00	0.00	0.00
85	85	85.00	0.00	0.00	0.00	0.00	0.00

# Microstran V9

Leaf 0  
Job: 0028 2015 Calverley House bottom 2 floor beams  
B & C More-Axels  
Floor beams

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08:33:43

104	85	8.00	0.32	0.30	3.04	0.83	-0.02
104	76	8.00	-1.74	0.00	3.04	0.83	-0.94
104	82	8.00	-1.74	0.00	3.04	0.83	-0.42
105	81	8.00	-0.88	0.00	3.04	0.83	-0.36
105	83	8.00	-0.88	0.00	3.04	0.83	-0.08
106	84	8.00	-2.10	0.00	3.04	0.83	-1.48
106	82	8.00	-2.10	0.00	3.04	0.83	-0.55
107	87	8.00	0.68	0.00	3.04	0.83	0.00
108	88	8.00	0.18	0.00	3.04	0.83	-0.02
108	89	-0.88	0.00	0.00	3.04	0.83	0.00
108	85	-0.88	0.00	0.00	3.04	0.83	0.00
109	81	8.00	-0.57	0.00	3.04	0.83	-0.03
109	81	8.00	0.84	0.00	3.04	0.83	0.30
110	82	-1.74	0.00	0.00	3.04	0.83	0.00
110	81	-1.74	0.00	0.00	3.04	0.83	-0.12
111	81	8.00	-0.46	0.00	3.04	0.83	-0.19
111	83	8.00	0.32	0.00	3.04	0.83	-0.07
112	82	-0.47	0.00	0.00	3.04	0.83	0.00
112	83	-0.47	0.00	0.00	3.04	0.83	0.00
113	82	8.00	-0.25	0.00	3.04	0.83	-0.01
113	81	8.00	-0.25	0.00	3.04	0.83	0.00
114	82	8.00	-1.89	0.00	3.04	0.83	-0.12
114	83	8.00	-1.89	0.00	3.04	0.83	0.00
115	89	8.00	-0.21	0.00	3.04	0.83	0.00
115	88	8.00	-0.21	0.00	3.04	0.83	0.00
116	90	8.00	-1.27	0.00	3.04	0.83	-0.55
116	89	8.00	-1.27	0.00	3.04	0.83	0.00
117	83	-1.74	0.00	0.00	3.04	0.83	0.00
117	82	-1.74	0.00	0.00	3.04	0.83	-0.12
118	84	8.00	0.07	0.00	3.04	0.83	0.00
118	84	8.00	0.07	0.00	3.04	0.83	-0.01
119	87	8.00	0.00	0.00	3.04	0.83	0.00
119	85	8.00	0.00	0.00	3.04	0.83	-0.01
120	93	8.00	-0.21	0.00	3.04	0.83	0.00
120	92	-0.21	0.00	0.00	3.04	0.83	0.00
121	89	8.00	-0.74	0.00	3.04	0.83	-0.12
121	88	8.00	-0.74	0.00	3.04	0.83	0.00
122	82	8.00	0.32	0.00	3.04	0.83	-0.12
122	83	-0.77	0.00	0.00	3.04	0.83	0.00
123	71	-1.74	0.00	0.00	3.04	0.83	0.00
123	72	8.00	-0.84	0.00	3.04	0.83	-0.12
123	82	8.00	-0.84	0.00	3.04	0.83	0.12
124	131	8.00	-0.15	0.00	3.04	0.83	0.12
124	3	8.00	0.61	0.00	3.04	0.83	-0.01
124	131	8.00	-0.85	0.00	3.04	0.83	-0.12
124	134	8.00	-0.74	0.00	3.04	0.83	-0.43
125	134	8.00	0.21	0.00	3.04	0.83	-0.12
125	132	8.00	0.81	0.00	3.04	0.83	-0.01
125	133	8.00	-0.22	0.00	3.04	0.83	0.00
126	124	8.00	0.07	0.00	3.04	0.83	0.00
126	125	8.00	0.07	0.00	3.04	0.83	0.00
127	111	8.00	0.00	0.00	3.04	0.83	0.00
127	128	8.00	0.00	0.00	3.04	0.83	0.00
127	129	8.00	0.00	0.00	3.04	0.83	0.00
128	117	8.00	0.10	0.00	3.04	0.83	0.00
128	114	8.00	0.00	0.00	3.04	0.83	0.00
128	125	8.00	0.00	0.00	3.04	0.83	0.00
128	126	8.00	0.00	0.00	3.04	0.83	0.00
128	127	8.00	0.00	0.00	3.04	0.83	0.00
128	128	8.00	0.00	0.00	3.04	0.83	0.00
128	129	8.00	0.00	0.00	3.04	0.83	0.00
128	130	8.00	0.00	0.00	3.04	0.83	0.00
128	131	8.00	0.00	0.00	3.04	0.83	0.00
128	132	8.00	0.00	0.00	3.04	0.83	0.00
128	133	8.00	0.00	0.00	3.04	0.83	0.00
128	134	8.00	0.00	0.00	3.04	0.83	0.00
128	135	8.00	0.00	0.00	3.04	0.83	0.00
128	136	8.00	0.00	0.00	3.04	0.83	0.00
128	137	8.00	0.00	0.00	3.04	0.83	0.00
128	138	8.00	0.00	0.00	3.04	0.83	0.00
128	139	8.00	0.00	0.00	3.04	0.83	0.00
128	140	8.00	0.00	0.00	3.04	0.83	0.00
128	141	8.00	0.00	0.00	3.04	0.83	0.00
128	142	8.00	0.00	0.00	3.04	0.83	0.00
128	143	8.00	0.00	0.00	3.04	0.83	0.00
128	144	8.00	0.00	0.00	3.04	0.83	0.00
128	145	8.00	0.00	0.00	3.04	0.83	0.00
128	146	8.00	0.00	0.00	3.04	0.83	0.00
128	147	8.00	0.00	0.00	3.04	0.83	0.00
128	148	8.00	0.00	0.00	3.04	0.83	0.00
128	149	8.00	0.00	0.00	3.04	0.83	0.00
128	150	8.00	0.00	0.00	3.04	0.83	0.00
128	151	8.00	0.00	0.00	3.04	0.83	0.00
128	152	8.00	0.00	0.00	3.04	0.83	0.00
128	153	8.00	0.00	0.00	3.04	0.83	0.00
128	154	8.00	0.00	0.00	3.04	0.83	0.00
128	155	8.00	0.00	0.00	3.04	0.83	0.00
128	156	8.00	0.00	0.00	3.04	0.83	0.00
128	157	8.00	0.00	0.00	3.04	0.83	0.00
128	158	8.00	0.00	0.00	3.04	0.83	0.00
128	159	8.00	0.00	0.00	3.04	0.83	0.00
128	160	8.00	0.00	0.00	3.04	0.83	0.00
128	161	8.00	0.00	0.00	3.04	0.83	0.00
128	162	8.00	0.00	0.00	3.04	0.83	0.00
128	163	8.00	0.00	0.00	3.04	0.83	0.00
128	164	8.00	0.00	0.00	3.04	0.83	0.00
128	165	8.00	0.00	0.00	3.04	0.83	0.00
128	166	8.00	0.00	0.00	3.04	0.83	0.00
128	167	8.00	0.00	0.00	3.04	0.83	0.00
128	168	8.00	0.00	0.00	3.04	0.83	0.00
128	169	8.00	0.00	0.00	3.04	0.83	0.00
128	170	8.00	0.00	0.00	3.04	0.83	0.00
128	171	8.00	0.00	0.00	3.04	0.83	0.00
128	172	8.00	0.00	0.00	3.04	0.83	0.00
128	173	8.00	0.00	0.00	3.04	0.83	0.00
128	174	8.00	0.00	0.00	3.04	0.83	0.00
128	175	8.00	0.00	0.00	3.04	0.83	0.00
128	176	8.00	0.00	0.00	3.04	0.83	0.00
128	177	8.00	0.00	0.00	3.04	0.83	0.00
128	178	8.00	0.00	0.00	3.04	0.83	0.00
128	179	8.00	0.00	0.00	3.04	0.83	0.00
128	180	8.00	0.00	0.00	3.04	0.83	0.00
128	181	8.00	0.00	0.00	3.04	0.83	0.00
128	182	8.00	0.00	0.00	3.04	0.83	0.00
128	183	8.00	0.00	0.00	3.04	0.83	0.00
128	184	8.00	0.00	0.00	3.04	0.83	0.00
128	185	8.00	0.00	0.00	3.04	0.83	0.00
128	186	8.00	0.00	0.00	3.04	0.83	0.00
128	187	8.00	0.00	0.00	3.04	0.83	0.00
128	188	8.00	0.00	0.00	3.04	0.83	0.00
128	189	8.00	0.00	0.00	3.04	0.83	0.00
128	190	8.00	0.00	0.00	3.04	0.83	0.00
128	191	8.00	0.00	0.00	3.04	0.83	0.00
128	192	8.00	0.00	0.00	3.04	0.83	0.00
128	193	8.00	0.00	0.00	3.04	0.83	0.00
128	194	8.00	0.00	0.00	3.04	0.83	0.00
128	195	8.00	0.00	0.00	3.04	0.83	0.00
128	196	8.00	0.00	0.00	3.04	0.83	0.00
128	197	8.00	0.00	0.00	3.04	0.83	0.00
128	198	8.00	0.00	0.00	3.04	0.83	0.00
128	199	8.00	0.00	0.00	3.04	0.83	0.00
128	200	8.00	0.00	0.00	3.04	0.83	0.00
128	201	8.00	0.00	0.00	3.04	0.83	0.00
128	202	8.00	0.00	0.00	3.04	0.83	0.00
128	203	8.00	0.00	0.00	3.04	0.83	0.00
128	204	8.00	0.00	0.00	3.04	0.83	0.00
128	205	8.00	0.00	0.00	3.04	0.83	0.00
128	206	8.00	0.00	0.00	3.04	0.83	0.00
128	207	8.00	0.00	0.00	3.04	0.83	0.00
128	208	8.00	0.00	0.00	3.04	0.83	0.00
128	209	8.00	0.00	0.00	3.04	0.83	0.00
128	210	8.00	0.00	0.00	3.04	0.83	0.00
128	211	8.00	0.00	0.00	3.04	0.83	0.00
128	212	8.00	0.00	0.00	3.04	0.83	0.00
128	213	8.00	0.00	0.00	3.04	0.83	0.00
128	214	8.00	0.00	0.00	3.04	0.83	0.00
128	215	8.00	0.00	0.00	3.04	0.83	0.00
128	216	8.00	0.00	0.00	3.04	0.83	0.00
128	217	8.00	0.00	0.00	3.04	0.83	0.00
128	218	8.00	0.00	0.00	3.04	0.83	0.00
128	219	8.00	0.00	0.00	3.04	0.83	0.00
128	220	8.00	0.00	0.00	3.04	0.83	0.00
128	221	8.00	0.00	0.00	3.04	0.83	0.00
128	222	8.00	0.00	0.00	3.04	0.83	0.00
128	223	8.00	0.00	0.00	3.04	0.83	0.00
128	224	8.00	0.00	0.00	3.04	0.83	0.00
128	225	8.00	0.00	0.00	3.04	0.83	0.00
128	226	8.00	0.00	0.00	3.04	0.83	0.00
128	227	8.00	0.00	0.00	3.04	0.83	0.00
128	228	8.00	0.00	0.00	3.04	0.83	0.00
128	229	8.00	0.00	0.00	3.04	0.83	0.00
128	230	8.00	0.00	0.00	3.04	0.83	0.00
128	231	8.00	0.00	0.00	3.04	0.83	



# Microstran V9

LYALLO  
 Job: 0508 2015 Catharine House Bedroom 3 floor beams  
 B & C MARK-NEW  
 Plan 05011

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## APPOINT DIMENSIONS

Case	6: lateral load X							
Node	Force-X	Force-Y	Force-X	Force-Y	Moment-X	Moment-Y	Moment-X	Moment-Y
	kN	kN	kN	kN	kNm	kNm	kNm	kNm
17	-7.82	-0.22	0.00	0.13	-3.37	3.68		
20	-3.18	0.28	0.00	-1.48	-0.38	3.02		
21	-3.45	0.28	0.00	-1.48	-1.95	3.02		
15	0.00	0.00	0.00	0.00	0.00	0.00		
70	-4.48	-1.72	0.00	0.88	-3.29	3.62		
30	-4.48	0.27	0.00	-0.92	-0.13	3.02		
13	-3.28	1.81	0.00	-1.41	-0.41	3.02		
102	-9.44	0.33	0.00	2.08	-0.89	3.68		
108	-9.48	0.33	0.00	2.08	-0.22	3.02		
109	-9.24	0.00	0.00	0.00	-0.32	3.02		
108	-8.40	0.00	0.00	0.00	-1.38	3.02		
108	-8.16	0.00	0.00	0.00	-3.41	3.02		
171	-8.44	0.00	0.00	0.00	-0.42	3.02		
100	-28.46	0.00	0.00	0.00				

Max. reaction: 7.576E+01 at 8099: 187  
 (Reaction set to structure in position global with direction.)

## Case 7: lateral load Y

Node	X Force	Y Force	X Moment	Y Moment	X Support	Y Support
	kN	kN	kNm	kNm	kN	kN
19	0.000	0.425	0.000	0.000	0.000	3.833
23	0.000	0.425	0.000	0.000	0.000	3.833
25	0.000	0.425	0.000	0.000	0.000	3.833
28	0.000	0.425	0.000	0.000	0.000	3.833
31	0.000	0.425	0.000	0.000	0.000	3.833
75	0.000	0.425	0.000	0.000	0.000	3.833
78	0.000	0.425	0.000	0.000	0.000	3.833
79	0.000	0.425	0.000	0.000	0.000	3.833
41	0.000	0.425	0.000	0.000	0.000	3.833
49	0.000	0.425	0.000	0.000	0.000	3.833
47	0.000	0.425	0.000	0.000	0.000	3.833
48	0.000	0.425	0.000	0.000	0.000	3.833
52	0.000	0.425	0.000	0.000	0.000	3.833
55	0.000	0.425	0.000	0.000	0.000	3.833
57	0.000	0.425	0.000	0.000	0.000	3.833
62	0.000	0.425	0.000	0.000	0.000	3.833
63	0.000	0.425	0.000	0.000	0.000	3.833
69	0.000	0.425	0.000	0.000	0.000	3.833
68	0.000	0.425	0.000	0.000	0.000	3.833
71	0.000	0.425	0.000	0.000	0.000	3.833
73	0.000	0.425	0.000	0.000	0.000	3.833
70	0.000	0.425	0.000	0.000	0.000	3.833
64	0.000	0.425	0.000	0.000	0.000	3.833
64	0.000	0.425	0.000	0.000	0.000	3.833

Top of Applied Loads (Global Axis):  
 (X) 0.000 (Y) 0.000  
 Moment about x-y global origin:  
 (X) -77.863 (Y) 0.000 (Z) 24.450

## JOINT DISPLACEMENTS

Case	7: lateral load Y					
Node	X-Disp	Y-Disp	Z-Disp	Rot-X	Rot-Y	Rot-Z
	mm	mm	mm	rad	rad	rad
1	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
2	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
3	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
4	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
5	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
6	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
7	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
8	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
9	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
10	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
11	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
12	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
13	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
14	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
15	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
16	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
17	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
18	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
19	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
20	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
21	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
22	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
23	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
24	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
25	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
26	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
27	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
28	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
29	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
30	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
31	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
32	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
33	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
34	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
35	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
36	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
37	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
38	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
39	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
40	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
41	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
42	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
43	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
44	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
45	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
46	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
47	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
48	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
49	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
50	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
51	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
52	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
53	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
54	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
55	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
56	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
57	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
58	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
59	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
60	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
61	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
62	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
63	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
64	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
65	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
66	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
67	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
68	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
69	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
70	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194

71	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
72	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
73	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
74	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
75	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
76	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
77	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
78	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
79	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
80	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
81	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
82	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
83	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
84	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
85	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
86	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
87	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
88	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
89	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
90	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
91	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
92	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
93	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
94	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
95	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
96	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
97	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
98	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
99	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
100	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
101	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
102	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
103	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
104	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
105	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
106	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
107	-0.0067	0.0082	0.0000	0.0000	0.0000	-0.00194
108	-0.0067					

# Microstran V9

Proj. 0  
 Job: 0003 2010 Catherine House bedroom 3 floor beams  
 B & D Martin-Jones  
 Floor beams

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 15/04/15  
 00:33:42 s.

38	0.00	0.00	0.00	0.00	0.00	0.00
39	0.00	0.00	0.00	0.00	0.00	0.00
40	0.00	0.00	0.00	0.00	0.00	0.00
41	0.00	0.00	0.00	0.00	0.00	0.00
42	0.00	0.00	0.00	0.00	0.00	0.00
43	0.00	0.00	0.00	0.00	0.00	0.00
44	0.00	0.00	0.00	0.00	0.00	0.00
45	0.00	0.00	0.00	0.00	0.00	0.00
46	0.00	0.00	0.00	0.00	0.00	0.00
47	0.00	0.00	0.00	0.00	0.00	0.00
48	0.00	0.00	0.00	0.00	0.00	0.00
49	0.00	0.00	0.00	0.00	0.00	0.00
50	0.00	0.00	0.00	0.00	0.00	0.00
51	0.00	0.00	0.00	0.00	0.00	0.00
52	0.00	0.00	0.00	0.00	0.00	0.00
53	0.00	0.00	0.00	0.00	0.00	0.00
54	0.00	0.00	0.00	0.00	0.00	0.00
55	0.00	0.00	0.00	0.00	0.00	0.00
56	0.00	0.00	0.00	0.00	0.00	0.00
57	0.00	0.00	0.00	0.00	0.00	0.00
58	0.00	0.00	0.00	0.00	0.00	0.00
59	0.00	0.00	0.00	0.00	0.00	0.00
60	0.00	0.00	0.00	0.00	0.00	0.00
61	0.00	0.00	0.00	0.00	0.00	0.00
62	0.00	0.00	0.00	0.00	0.00	0.00
63	0.00	0.00	0.00	0.00	0.00	0.00
64	0.00	0.00	0.00	0.00	0.00	0.00
65	0.00	0.00	0.00	0.00	0.00	0.00
66	0.00	0.00	0.00	0.00	0.00	0.00
67	0.00	0.00	0.00	0.00	0.00	0.00
68	0.00	0.00	0.00	0.00	0.00	0.00
69	0.00	0.00	0.00	0.00	0.00	0.00
70	0.00	0.00	0.00	0.00	0.00	0.00
71	0.00	0.00	0.00	0.00	0.00	0.00
72	0.00	0.00	0.00	0.00	0.00	0.00
73	0.00	0.00	0.00	0.00	0.00	0.00
74	0.00	0.00	0.00	0.00	0.00	0.00
75	0.00	0.00	0.00	0.00	0.00	0.00
76	0.00	0.00	0.00	0.00	0.00	0.00
77	0.00	0.00	0.00	0.00	0.00	0.00
78	0.00	0.00	0.00	0.00	0.00	0.00
79	0.00	0.00	0.00	0.00	0.00	0.00
80	0.00	0.00	0.00	0.00	0.00	0.00
81	0.00	0.00	0.00	0.00	0.00	0.00
82	0.00	0.00	0.00	0.00	0.00	0.00
83	0.00	0.00	0.00	0.00	0.00	0.00
84	0.00	0.00	0.00	0.00	0.00	0.00
85	0.00	0.00	0.00	0.00	0.00	0.00
86	0.00	0.00	0.00	0.00	0.00	0.00
87	0.00	0.00	0.00	0.00	0.00	0.00
88	0.00	0.00	0.00	0.00	0.00	0.00
89	0.00	0.00	0.00	0.00	0.00	0.00
90	0.00	0.00	0.00	0.00	0.00	0.00
91	0.00	0.00	0.00	0.00	0.00	0.00
92	0.00	0.00	0.00	0.00	0.00	0.00
93	0.00	0.00	0.00	0.00	0.00	0.00
94	0.00	0.00	0.00	0.00	0.00	0.00
95	0.00	0.00	0.00	0.00	0.00	0.00
96	0.00	0.00	0.00	0.00	0.00	0.00
97	0.00	0.00	0.00	0.00	0.00	0.00
98	0.00	0.00	0.00	0.00	0.00	0.00
99	0.00	0.00	0.00	0.00	0.00	0.00
100	0.00	0.00	0.00	0.00	0.00	0.00
101	0.00	0.00	0.00	0.00	0.00	0.00
102	0.00	0.00	0.00	0.00	0.00	0.00
103	0.00	0.00	0.00	0.00	0.00	0.00
104	0.00	0.00	0.00	0.00	0.00	0.00
105	0.00	0.00	0.00	0.00	0.00	0.00
106	0.00	0.00	0.00	0.00	0.00	0.00
107	0.00	0.00	0.00	0.00	0.00	0.00

108	0.00	0.00	0.00	0.00	0.00	0.00
109	0.00	0.00	0.00	0.00	0.00	0.00
110	0.00	0.00	0.00	0.00	0.00	0.00
111	0.00	0.00	0.00	0.00	0.00	0.00
112	0.00	0.00	0.00	0.00	0.00	0.00
113	0.00	0.00	0.00	0.00	0.00	0.00
114	0.00	0.00	0.00	0.00	0.00	0.00
115	0.00	0.00	0.00	0.00	0.00	0.00
116	0.00	0.00	0.00	0.00	0.00	0.00
117	0.00	0.00	0.00	0.00	0.00	0.00
118	0.00	0.00	0.00	0.00	0.00	0.00
119	0.00	0.00	0.00	0.00	0.00	0.00
120	0.00	0.00	0.00	0.00	0.00	0.00
121	0.00	0.00	0.00	0.00	0.00	0.00
122	0.00	0.00	0.00	0.00	0.00	0.00
123	0.00	0.00	0.00	0.00	0.00	0.00
124	0.00	0.00	0.00	0.00	0.00	0.00
125	0.00	0.00	0.00	0.00	0.00	0.00
126	0.00	0.00	0.00	0.00	0.00	0.00
127	0.00	0.00	0.00	0.00	0.00	0.00
128	0.00	0.00	0.00	0.00	0.00	0.00
129	0.00	0.00	0.00	0.00	0.00	0.00
130	0.00	0.00	0.00	0.00	0.00	0.00
131	0.00	0.00	0.00	0.00	0.00	0.00
132	0.00	0.00	0.00	0.00	0.00	0.00
133	0.00	0.00	0.00	0.00	0.00	0.00
134	0.00	0.00	0.00	0.00	0.00	0.00
135	0.00	0.00	0.00	0.00	0.00	0.00
136	0.00	0.00	0.00	0.00	0.00	0.00
137	0.00	0.00	0.00	0.00	0.00	0.00
138	0.00	0.00	0.00	0.00	0.00	0.00
139	0.00	0.00	0.00	0.00	0.00	0.00
140	0.00	0.00	0.00	0.00	0.00	0.00
141	0.00	0.00	0.00	0.00	0.00	0.00
142	0.00	0.00	0.00	0.00	0.00	0.00
143	0.00	0.00	0.00	0.00	0.00	0.00
144	0.00	0.00	0.00	0.00	0.00	0.00
145	0.00	0.00	0.00	0.00	0.00	0.00
146	0.00	0.00	0.00	0.00	0.00	0.00
147	0.00	0.00	0.00	0.00	0.00	0.00
148	0.00	0.00	0.00	0.00	0.00	0.00
149	0.00	0.00	0.00	0.00	0.00	0.00
150	0.00	0.00	0.00	0.00	0.00	0.00
151	0.00	0.00	0.00	0.00	0.00	0.00
152	0.00	0.00	0.00	0.00	0.00	0.00
153	0.00	0.00	0.00	0.00	0.00	0.00
154	0.00	0.00	0.00	0.00	0.00	0.00
155	0.00	0.00	0.00	0.00	0.00	0.00
156	0.00	0.00	0.00	0.00	0.00	0.00
157	0.00	0.00	0.00	0.00	0.00	0.00
158	0.00	0.00	0.00	0.00	0.00	0.00
159	0.00	0.00	0.00	0.00	0.00	0.00
160	0.00	0.00	0.00	0.00	0.00	0.00
161	0.00	0.00	0.00	0.00	0.00	0.00
162	0.00	0.00	0.00	0.00	0.00	0.00
163	0.00	0.00	0.00	0.00	0.00	0.00
164	0.00	0.00	0.00	0.00	0.00	0.00

Positive Moments (kNm):  
 Left - Positive  
 Top - Right-hand side  
 Bottom - Sailing

Support Reactions

Case: 1: 1,000 Dead  
 2: 1,000 Live

Max. Displacement: -1.2480-13 at 2009: 167

(Reactions are in thousands in positive global axis directions.)

Case: 1: 0 4 2

Node	Reaction	Value
17	17	3.33
20	20	0.33
21	21	0.33
70	70	-3.37
80	80	-0.37
81	81	-0.37
100	100	0.00
108	108	0.00
109	109	0.00
140	140	0.00
148	148	0.00
171	171	0.00

Node Displacements

Case: 1: 0 4 2

Node	Z-Disp	Y-Disp	X-Disp	Y-Rot	Z-Rot	X-Rot
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
3	0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
4	0.0000	0.0000	-0.0000	0.0000	0.0000	0.0000
5	0.0001	0.0000	-0.0000	0.0000	0.0000	0.0000
6	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
7	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
8	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
9	0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
10	0.0000	0.0000	-0.0000	0.0000	0.0000	0.0000
11	0.0001	0.0000	-0.0000	0.0000	0.0000	0.0000
12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
13	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
14	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
15	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
16	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
17	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
18	0.0000	0.0000	-0.0000	0.0000	0.0000	0.0000
19	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
20	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
21	0.0001	0.0000	-0.0000	0.0000	0.0000	0.0000
22	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
23	0.0001	0.0000	-0.0000	0.0000	0.0000	0.0000
24	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
25	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
26	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000
27	0.0001	-0.0002	-0.0001	0.0000	0.0000	0.0000





# Microstran V9

Level: 0  
 Job: 3005 2013 Calverley House between 3 floor beams  
 B & C Motion-Jones  
 Floor beams

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 15/04/13  
 08:33:43

Node	X	Y	Z	U	V	W	TX	TY	TZ
67	71	3.02	0.89	0.00	0.00	0.00	-0.20		
67	71	-1.03	0.89	0.00	0.00	0.00	0.00		
68	72	3.08	0.74	0.00	0.00	0.00	0.00		
68	72	-1.03	0.74	0.00	0.00	0.00	0.00		
69	75	-1.44	0.23	0.00	0.00	0.00	0.00		
69	75	-1.88	0.23	0.00	0.00	0.00	0.00		
70	74	3.00	-0.89	0.00	0.00	0.00	-0.11		
70	74	-1.00	-0.89	0.00	0.00	0.00	0.00		
71	60	3.04	4.17	0.00	0.00	0.00	-1.27		
71	60	-1.00	4.17	0.00	0.00	0.00	0.00		
72	71	3.02	4.87	0.00	0.00	0.00	0.00		
72	71	-1.00	4.87	0.00	0.00	0.00	-0.18		
73	75	3.02	2.44	0.00	0.00	0.00	-0.18		
73	75	-1.00	2.44	0.00	0.00	0.00	0.00		
74	78	-11.51	0.80	0.00	0.00	0.00	0.00		
74	78	-11.91	0.80	0.00	0.00	0.00	0.00		
75	74	-1.10	0.89	-0.31	0.00	0.00	0.00		
75	74	-1.10	0.89	-0.31	0.00	0.00	0.00		
76	77	3.00	0.23	0.00	0.00	0.00	0.00		
76	77	-1.00	0.23	0.00	0.00	0.00	0.00		
77	79	0.90	0.70	0.00	0.00	0.00	0.00		
77	79	-0.90	0.70	0.00	0.00	0.00	0.00		
78	82	0.90	0.89	0.00	0.00	0.00	0.00		
78	82	-0.90	0.89	0.00	0.00	0.00	0.00		
79	81	-12.87	0.00	0.00	0.00	0.00	0.00		
79	81	-12.87	0.00	0.00	0.00	0.00	0.00		
80	81	-1.14	0.00	0.00	0.00	0.00	0.00		
80	81	-1.14	0.00	0.00	0.00	0.00	0.00		
81	82	0.90	0.00	0.00	0.00	0.00	0.00		
81	82	-0.90	0.00	0.00	0.00	0.00	0.00		
82	83	0.90	0.10	0.00	0.00	0.00	0.00		
82	83	-0.90	0.10	0.00	0.00	0.00	0.00		
83	83	-14.69	0.00	0.00	0.00	0.00	0.00		
83	83	-14.69	0.00	0.00	0.00	0.00	0.00		
84	84	-1.14	0.00	0.00	0.00	0.00	0.00		
84	84	-1.14	0.00	0.00	0.00	0.00	0.00		
85	85	-1.14	0.00	0.00	0.00	0.00	0.00		
85	85	-1.14	0.00	0.00	0.00	0.00	0.00		
86	86	0.90	0.10	0.00	0.00	0.00	0.00		
86	86	-0.90	0.10	0.00	0.00	0.00	0.00		
87	87	0.90	0.10	0.00	0.00	0.00	0.00		
87	87	-0.90	0.10	0.00	0.00	0.00	0.00		
88	88	0.90	0.10	0.00	0.00	0.00	0.00		
88	88	-0.90	0.10	0.00	0.00	0.00	0.00		
89	89	0.90	0.10	0.00	0.00	0.00	0.00		
89	89	-0.90	0.10	0.00	0.00	0.00	0.00		
90	90	0.90	0.10	0.00	0.00	0.00	0.00		
90	90	-0.90	0.10	0.00	0.00	0.00	0.00		
91	91	0.90	0.10	0.00	0.00	0.00	0.00		
91	91	-0.90	0.10	0.00	0.00	0.00	0.00		
92	92	0.90	0.10	0.00	0.00	0.00	0.00		
92	92	-0.90	0.10	0.00	0.00	0.00	0.00		
93	93	0.90	0.10	0.00	0.00	0.00	0.00		
93	93	-0.90	0.10	0.00	0.00	0.00	0.00		
94	94	0.90	0.10	0.00	0.00	0.00	0.00		
94	94	-0.90	0.10	0.00	0.00	0.00	0.00		
95	95	0.90	0.10	0.00	0.00	0.00	0.00		
95	95	-0.90	0.10	0.00	0.00	0.00	0.00		
96	96	0.90	0.10	0.00	0.00	0.00	0.00		
96	96	-0.90	0.10	0.00	0.00	0.00	0.00		
97	97	0.90	0.10	0.00	0.00	0.00	0.00		
97	97	-0.90	0.10	0.00	0.00	0.00	0.00		
98	98	0.90	0.10	0.00	0.00	0.00	0.00		
98	98	-0.90	0.10	0.00	0.00	0.00	0.00		
99	99	0.90	0.10	0.00	0.00	0.00	0.00		
99	99	-0.90	0.10	0.00	0.00	0.00	0.00		
100	100	0.90	0.10	0.00	0.00	0.00	0.00		
100	100	-0.90	0.10	0.00	0.00	0.00	0.00		
101	101	0.90	0.10	0.00	0.00	0.00	0.00		
101	101	-0.90	0.10	0.00	0.00	0.00	0.00		
102	102	0.90	0.10	0.00	0.00	0.00	0.00		
102	102	-0.90	0.10	0.00	0.00	0.00	0.00		
103	103	0.90	0.10	0.00	0.00	0.00	0.00		
103	103	-0.90	0.10	0.00	0.00	0.00	0.00		
104	104	0.90	0.10	0.00	0.00	0.00	0.00		
104	104	-0.90	0.10	0.00	0.00	0.00	0.00		
105	105	0.90	0.10	0.00	0.00	0.00	0.00		
105	105	-0.90	0.10	0.00	0.00	0.00	0.00		
106	106	0.90	0.10	0.00	0.00	0.00	0.00		
106	106	-0.90	0.10	0.00	0.00	0.00	0.00		
107	107	0.90	0.10	0.00	0.00	0.00	0.00		
107	107	-0.90	0.10	0.00	0.00	0.00	0.00		
108	108	0.90	0.10	0.00	0.00	0.00	0.00		
108	108	-0.90	0.10	0.00	0.00	0.00	0.00		
109	109	0.90	0.10	0.00	0.00	0.00	0.00		
109	109	-0.90	0.10	0.00	0.00	0.00	0.00		
110	110	0.90	0.10	0.00	0.00	0.00	0.00		
110	110	-0.90	0.10	0.00	0.00	0.00	0.00		
111	111	0.90	0.10	0.00	0.00	0.00	0.00		
111	111	-0.90	0.10	0.00	0.00	0.00	0.00		
112	112	0.90	0.10	0.00	0.00	0.00	0.00		
112	112	-0.90	0.10	0.00	0.00	0.00	0.00		
113	113	0.90	0.10	0.00	0.00	0.00	0.00		
113	113	-0.90	0.10	0.00	0.00	0.00	0.00		
114	114	0.90	0.10	0.00	0.00	0.00	0.00		
114	114	-0.90	0.10	0.00	0.00	0.00	0.00		
115	115	0.90	0.10	0.00	0.00	0.00	0.00		
115	115	-0.90	0.10	0.00	0.00	0.00	0.00		
116	116	0.90	0.10	0.00	0.00	0.00	0.00		
116	116	-0.90	0.10	0.00	0.00	0.00	0.00		
117	117	0.90	0.10	0.00	0.00	0.00	0.00		
117	117	-0.90	0.10	0.00	0.00	0.00	0.00		
118	118	0.90	0.10	0.00	0.00	0.00	0.00		
118	118	-0.90	0.10	0.00	0.00	0.00	0.00		
119	119	0.90	0.10	0.00	0.00	0.00	0.00		
119	119	-0.90	0.10	0.00	0.00	0.00	0.00		
120	120	0.90	0.10	0.00	0.00	0.00	0.00		
120	120	-0.90	0.10	0.00	0.00	0.00	0.00		
121	121	0.90	0.10	0.00	0.00	0.00	0.00		
121	121	-0.90	0.10	0.00	0.00	0.00	0.00		
122	122	0.90	0.10	0.00	0.00	0.00	0.00		
122	122	-0.90	0.10	0.00	0.00	0.00	0.00		
123	123	0.90	0.10	0.00	0.00	0.00	0.00		
123	123	-0.90	0.10	0.00	0.00	0.00	0.00		
124	124	0.90	0.10	0.00	0.00	0.00	0.00		
124	124	-0.90	0.10	0.00	0.00	0.00	0.00		
125	125	0.90	0.10	0.00	0.00	0.00	0.00		
125	125	-0.90	0.10	0.00	0.00	0.00	0.00		
126	126	0.90	0.10	0.00	0.00	0.00	0.00		
126	126	-0.90	0.10	0.00	0.00	0.00	0.00		
127	127	0.90	0.10	0.00	0.00	0.00	0.00		
127	127	-0.90	0.10	0.00	0.00	0.00	0.00		
128	128	0.90	0.10	0.00	0.00	0.00	0.00		
128	128	-0.90	0.10	0.00	0.00	0.00	0.00		
129	129	0.90	0.10	0.00	0.00	0.00	0.00		
129	129	-0.90	0.10	0.00	0.00	0.00	0.00		
130	130	0.90	0.10	0.00	0.00	0.00	0.00		
130	130	-0.90	0.10	0.00	0.00	0.00	0.00		
131	131	0.90	0.10	0.00	0.00	0.00	0.00		
131	131	-0.90	0.10	0.00	0.00	0.00	0.00		
132	132	0.90	0.10	0.00	0.00	0.00	0.00		
132	132	-0.90	0.10	0.00	0.00	0.00	0.00		
133	133	0.90	0.10	0.00	0.00	0.00	0.00		
133	133	-0.90	0.10	0.00	0.00	0.00	0.00		
134	134	0.90	0.10	0.00	0.00	0.00	0.00		
134	134	-0.90	0.10	0.00	0.00	0.00	0.00		
135	135	0.90	0.10	0.00	0.00	0.00	0.00		
135	135	-0.90	0.10	0.00	0.00	0.00	0.00		
136	136	0.90	0.10	0.00	0.00	0.00	0.00		
136	136	-0.90	0.10	0.00	0.00	0.00	0.00		
137	137	0.90	0.10	0.00	0.00	0.00	0.00		
137	137	-0.90	0.10	0.00	0.00	0.00	0.00		

Positive (Down) Global Axis:  
 Axial - Positive  
 Torque - Right-hand twist

Shear - Zed Sipping  
 Moment - Negative

### SUPPORT REACTIONS

Node	Reaction	Value	Unit
17	Reaction	0.00	22.09
20	Reaction	0.00	20.41
23	Reaction	0.00	11.18
26	Reaction	0.00	9.08
29	Reaction	-	

# Microstran V9

Level 0  
 Job: 0000 2015 California Hours bedrock 3. Four beams  
 S & C Nelson-Jones  
 Four beams

Page 12 of 14  
 15/01/15  
 08:31:43 a.

## MEMBER FORCES

CASE: 41 26 1 0

Member	Node	Local	Shear	Moment	Torque	Rotational	Rotational
		1	2	3	4	5	6
1	1	-0.38	0.00	-0.00	0.00	0.00	0.00
1	6	-0.38	0.00	-0.00	0.00	0.00	0.00
2	2	0.00	1.50	0.00	0.00	0.00	0.00
3	3	0.00	1.50	0.00	0.00	0.00	0.00
4	4	0.00	1.50	0.00	0.00	0.00	0.00
5	5	0.00	1.50	0.00	0.00	0.00	0.00
6	6	-0.38	0.00	-0.00	0.00	0.00	0.00
7	7	-0.38	0.00	-0.00	0.00	0.00	0.00
8	8	0.00	1.50	0.00	0.00	0.00	0.00
9	9	0.00	1.50	0.00	0.00	0.00	0.00
10	10	0.00	1.50	0.00	0.00	0.00	0.00
11	11	0.00	1.50	0.00	0.00	0.00	0.00
12	12	0.00	1.50	0.00	0.00	0.00	0.00
13	13	0.00	1.50	0.00	0.00	0.00	0.00
14	14	0.00	1.50	0.00	0.00	0.00	0.00
15	15	0.00	1.50	0.00	0.00	0.00	0.00
16	16	0.00	1.50	0.00	0.00	0.00	0.00
17	17	0.00	1.50	0.00	0.00	0.00	0.00
18	18	0.00	1.50	0.00	0.00	0.00	0.00
19	19	0.00	1.50	0.00	0.00	0.00	0.00
20	20	0.00	1.50	0.00	0.00	0.00	0.00
21	21	0.00	1.50	0.00	0.00	0.00	0.00
22	22	0.00	1.50	0.00	0.00	0.00	0.00
23	23	0.00	1.50	0.00	0.00	0.00	0.00
24	24	0.00	1.50	0.00	0.00	0.00	0.00
25	25	0.00	1.50	0.00	0.00	0.00	0.00
26	26	0.00	1.50	0.00	0.00	0.00	0.00
27	27	0.00	1.50	0.00	0.00	0.00	0.00
28	28	0.00	1.50	0.00	0.00	0.00	0.00
29	29	0.00	1.50	0.00	0.00	0.00	0.00
30	30	0.00	1.50	0.00	0.00	0.00	0.00
31	31	0.00	1.50	0.00	0.00	0.00	0.00
32	32	0.00	1.50	0.00	0.00	0.00	0.00
33	33	0.00	1.50	0.00	0.00	0.00	0.00
34	34	0.00	1.50	0.00	0.00	0.00	0.00
35	35	0.00	1.50	0.00	0.00	0.00	0.00
36	36	0.00	1.50	0.00	0.00	0.00	0.00
37	37	0.00	1.50	0.00	0.00	0.00	0.00
38	38	0.00	1.50	0.00	0.00	0.00	0.00
39	39	0.00	1.50	0.00	0.00	0.00	0.00
40	40	0.00	1.50	0.00	0.00	0.00	0.00
41	41	0.00	1.50	0.00	0.00	0.00	0.00
42	42	0.00	1.50	0.00	0.00	0.00	0.00
43	43	0.00	1.50	0.00	0.00	0.00	0.00
44	44	0.00	1.50	0.00	0.00	0.00	0.00
45	45	0.00	1.50	0.00	0.00	0.00	0.00
46	46	0.00	1.50	0.00	0.00	0.00	0.00
47	47	0.00	1.50	0.00	0.00	0.00	0.00
48	48	0.00	1.50	0.00	0.00	0.00	0.00
49	49	0.00	1.50	0.00	0.00	0.00	0.00
50	50	0.00	1.50	0.00	0.00	0.00	0.00
51	51	0.00	1.50	0.00	0.00	0.00	0.00
52	52	0.00	1.50	0.00	0.00	0.00	0.00
53	53	0.00	1.50	0.00	0.00	0.00	0.00
54	54	0.00	1.50	0.00	0.00	0.00	0.00
55	55	0.00	1.50	0.00	0.00	0.00	0.00
56	56	0.00	1.50	0.00	0.00	0.00	0.00
57	57	0.00	1.50	0.00	0.00	0.00	0.00
58	58	0.00	1.50	0.00	0.00	0.00	0.00
59	59	0.00	1.50	0.00	0.00	0.00	0.00
60	60	0.00	1.50	0.00	0.00	0.00	0.00
61	61	0.00	1.50	0.00	0.00	0.00	0.00
62	62	0.00	1.50	0.00	0.00	0.00	0.00
63	63	0.00	1.50	0.00	0.00	0.00	0.00
64	64	0.00	1.50	0.00	0.00	0.00	0.00
65	65	0.00	1.50	0.00	0.00	0.00	0.00
66	66	0.00	1.50	0.00	0.00	0.00	0.00
67	67	0.00	1.50	0.00	0.00	0.00	0.00
68	68	0.00	1.50	0.00	0.00	0.00	0.00
69	69	0.00	1.50	0.00	0.00	0.00	0.00
70	70	0.00	1.50	0.00	0.00	0.00	0.00
71	71	0.00	1.50	0.00	0.00	0.00	0.00
72	72	0.00	1.50	0.00	0.00	0.00	0.00
73	73	0.00	1.50	0.00	0.00	0.00	0.00
74	74	0.00	1.50	0.00	0.00	0.00	0.00
75	75	0.00	1.50	0.00	0.00	0.00	0.00
76	76	0.00	1.50	0.00	0.00	0.00	0.00
77	77	0.00	1.50	0.00	0.00	0.00	0.00
78	78	0.00	1.50	0.00	0.00	0.00	0.00
79	79	0.00	1.50	0.00	0.00	0.00	0.00
80	80	0.00	1.50	0.00	0.00	0.00	0.00
81	81	0.00	1.50	0.00	0.00	0.00	0.00
82	82	0.00	1.50	0.00	0.00	0.00	0.00
83	83	0.00	1.50	0.00	0.00	0.00	0.00
84	84	0.00	1.50	0.00	0.00	0.00	0.00
85	85	0.00	1.50	0.00	0.00	0.00	0.00
86	86	0.00	1.50	0.00	0.00	0.00	0.00
87	87	0.00	1.50	0.00	0.00	0.00	0.00
88	88	0.00	1.50	0.00	0.00	0.00	0.00
89	89	0.00	1.50	0.00	0.00	0.00	0.00
90	90	0.00	1.50	0.00	0.00	0.00	0.00
91	91	0.00	1.50	0.00	0.00	0.00	0.00
92	92	0.00	1.50	0.00	0.00	0.00	0.00
93	93	0.00	1.50	0.00	0.00	0.00	0.00
94	94	0.00	1.50	0.00	0.00	0.00	0.00
95	95	0.00	1.50	0.00	0.00	0.00	0.00
96	96	0.00	1.50	0.00	0.00	0.00	0.00
97	97	0.00	1.50	0.00	0.00	0.00	0.00
98	98	0.00	1.50	0.00	0.00	0.00	0.00
99	99	0.00	1.50	0.00	0.00	0.00	0.00
100	100	0.00	1.50	0.00	0.00	0.00	0.00

81	81	0.00	1.47	0.00	0.00	0.00	-0.20
82	82	0.00	1.49	0.00	0.00	0.00	-0.20
83	83	0.00	1.42	0.00	0.00	0.00	-0.18
84	84	-0.50	0.00	0.00	0.00	0.00	0.00
85	85	-0.50	0.00	0.00	0.00	0.00	0.00
86	86	0.00	0.00	0.00	0.00	0.00	0.00
87	87	0.00	0.00	0.00	0.00	0.00	0.00
88	88	0.00	0.00	0.00	0.00	0.00	0.00
89	89	-0.10	0.00	0.00	0.00	0.00	0.00
90	90	-0.10	0.00	0.00	0.00	0.00	0.00
91	91	0.00	0.00	0.00	0.00	0.00	0.00
92	92	0.00	0.00	0.00	0.00	0.00	0.00
93	93	0.00	0.00	0.00	0.00	0.00	0.00
94	94	0.00	0.00	0.00	0.00	0.00	0.00
95	95	0.00	0.00	0.00	0.00	0.00	0.00
96	96	0.00	0.00	0.00	0.00	0.00	0.00
97	97	0.00	0.00	0.00	0.00	0.00	0.00
98	98	0.00	0.00	0.00	0.00	0.00	0.00
99	99	0.00	0.00	0.00	0.00	0.00	0.00
100	100	0.00	0.00	0.00	0.00	0.00	0.00

# Microstran V9

Level 0  
 Job: 0000 2015 California House between 3 floor beams  
 E & C House-0000

Page 13 of 14  
 003343.n

## Node Items

Node	166	171	177	189	191	199	200	208	210	211	214
166	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
171	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
177	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
189	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
191	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
199	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
200	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
208	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
210	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
211	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
214	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

## Positive Forces (Member Axis):

Actual - Tension  
 Torque - Right-hand twist

Shear - End A supporting  
 Moment - Sagging

## MEMBER PROPERTIES

Case: 41 20 + 0

Node	Force-X	Force-Y	Force-Z	Moment-X	Moment-Y	Moment-Z
Node	EA	EA	EA	EM	EM	EM
11	0.00	0.00	19.47	0.00	0.00	0.00
20	0.00	0.00	17.99	0.00	0.00	0.00
21	0.00	0.00	18.13	0.00	0.00	0.00
42	0.00	0.00	0.00	0.00	0.00	0.00
88	0.00	-0.01	19.18	0.00	0.00	0.00
100	0.00	0.00	17.88	0.00	0.00	0.00
83	0.00	0.00	19.95	0.00	0.00	0.00
109	0.00	0.00	0.00	0.00	0.00	0.00
108	0.00	0.00	0.00	0.00	0.00	0.00
105	0.00	0.00	0.00	0.00	0.00	0.00
104	0.00	0.00	0.00	0.00	0.00	0.00
103	0.00	0.00	0.00	0.00	0.00	0.00
102	0.00	0.00	0.00	0.00	0.00	0.00
101	0.00	0.00	0.00	0.00	0.00	0.00
100	0.00	0.00	0.00	0.00	0.00	0.00
99	0.00	0.00	0.00	0.00	0.00	0.00
98	0.00	0.00	0.00	0.00	0.00	0.00
97	0.00	0.00	0.00	0.00	0.00	0.00
96	0.00	0.00	0.00	0.00	0.00	0.00
95	0.00	0.00	0.00	0.00	0.00	0.00
94	0.00	0.00	0.00	0.00	0.00	0.00
93	0.00	0.00	0.00	0.00	0.00	0.00
92	0.00	0.00	0.00	0.00	0.00	0.00
91	0.00	0.00	0.00	0.00	0.00	0.00
90	0.00	0.00	0.00	0.00	0.00	0.00
89	0.00	0.00	0.00	0.00	0.00	0.00
88	0.00	0.00	0.00	0.00	0.00	0.00
87	0.00	0.00	0.00	0.00	0.00	0.00
86	0.00	0.00	0.00	0.00	0.00	0.00
85	0.00	0.00	0.00	0.00	0.00	0.00
84	0.00	0.00	0.00	0.00	0.00	0.00
83	0.00	0.00	0.00	0.00	0.00	0.00
82	0.00	0.00	0.00	0.00	0.00	0.00
81	0.00	0.00	0.00	0.00	0.00	0.00
80	0.00	0.00	0.00	0.00	0.00	0.00
79	0.00	0.00	0.00	0.00	0.00	0.00
78	0.00	0.00	0.00	0.00	0.00	0.00
77	0.00	0.00	0.00	0.00	0.00	0.00

(Moments are as structures in positive global axis directions.)

Case: 1 1.20 + 1.50

## Load Descriptions

Case: 1 1.20 Dead

2 1.50 Live

3 1.20 Wind

4 1.50 Live

5 1.20 Wind

6 1.50 Live

7 1.20 Wind

8 1.50 Live

9 1.20 Wind

10 1.50 Live

11 1.20 Wind

12 1.50 Live

13 1.20 Wind

14 1.50 Live

15 1.20 Wind

16 1.50 Live

17 1.20 Wind

18 1.50 Live

19 1.20 Wind

20 1.50 Live

21 1.20 Wind

22 1.50 Live

23 1.20 Wind

24 1.50 Live

25 1.20 Wind

26 1.50 Live

27 1.20 Wind

28 1.50 Live

29 1.20 Wind

30 1.50 Live

31 1.20 Wind

32 1.50 Live

33 1.20 Wind

34 1.50 Live

35 1.20 Wind

36 1.50 Live

37 1.20 Wind

38 1.50 Live

39 1.20 Wind

40 1.50 Live

41 1.20 Wind

42 1.50 Live

43 1.20 Wind

44 1.50 Live

45 1.20 Wind

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61 1.20 Wind

62 1.50 Live

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64 1.50 Live

65 1.20 Wind

66 1.50 Live

67 1.20 Wind

68 1.50 Live

69 1.20 Wind

70 1.50 Live

71 1.20 Wind

72 1.50 Live

73 1.20 Wind

74 1.50 Live

75 1.20 Wind

76 1.50 Live

Node	Force-X	Force-Y	Force-Z	Moment-X	Moment-Y	Moment-Z
77	0.0001	0.0001	4.9520E8	1.432E+08	-0.0002	0.0000
78	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
79	0.0001	-0.0001	-0.0001	1.432E+08	0.0000	0.0000
80	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
81	0.0000	0.0000	-0.0002	0.0000	0.0000	0.0000
82	0.0001	-0.0001	-0.0002	1.432E+08	0.0000	0.0000
83	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
84	0.0000	0.0000	-0.0000	-0.0000	0.0000	0.0000
85	0.0001	0.0001	-0.0002	1.432E+08	0.0000	0.0000
86	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
87	0.0001	-0.0001	0.0000	-0.0002	0.0000	0.0000
88	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
89	0.0001	0.0000	0.0000	-0.0002	0.0000	0.0000
90	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
91	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
92	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
93	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
94	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
95	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
96	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
97	0.0001	0.0001	0.0000	0.0000	0.0000	0.0000
98	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
99	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
100	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
101	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
102	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
103	0.0000	-0.0002	-0.0002	-1.432E+08	-0.0002	0.0000
104	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
105	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
106	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
107	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
108	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
109	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
110	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
111	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
112	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
113	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
114	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
115	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
116	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
117	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
118	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
119	0.0000	0.0000	0.0000	0.0000		





## Demand Calculation Sheet

single storey

V06/11

### Job Details

Name: Morron-Jones Residence  
 Street and Number: 52 Kauri Point Road, Laingholm  
 Lot and DP Number:  
 City/Town/District: Laingholm  
 Designer: A Barrett  
 Company Name: The draughting machine  
 Date: 15/10/2013



Select Lining Option

10 or 13 mm GIB® Plasterboard

### Building Specification

Number of storeys	single		
Floor Loading	2kPa		
Foundation Type	subfloor		
Cladding Weight (subfloor)	light		
		<b>Single Floor</b>	<b>Complete Single Floor Column only</b>
Cladding Weight	light		
Roof Weight	light		
Room in Roof Space	no		
Roof Pitch (degrees)	45		
Roof height above eaves (m)	2.0		
Building height to apex (m)	7.2		
Ground to lower floor level (m)	3.8		check heights, storey less than 2 m
Stud Height (m)	2.7		
Building Length (m)	8.2		
Building Width (m)	5.0		
Building Plan Area (m2)	38		

### Building Location

<b>Wind Zone</b>	High	<b>Earthquake Zone</b>	1	<b>Soil Type</b>	D&E (deep to very soft)
Select by Building Consent Authority Map or Preference	High	Annual exceedance probability	1/500 (NZS3604:2011 default)		
Wind Region	Preference selected				
Lee Zone	Preference selected				
Ground Roughness	Preference selected				
Site Exposure	Preference selected				
Topographic Class	Preference selected				

### Bracing Units required for Wind

Demand W (BU)		
	subfloor	Walls single
along	595	235
across	860	315

### Bracing Units required for Earthquake

Demand along / across E (BU)	
	Walls
	single
subfloor	276
single	138

**GIB EzyBrace® 2011 Software**



**SINGLE OR UPPER STOREY WALLS ALONG**

V06/11

Lines		Bracing Elements							
1	2	3	4	5	6	7	8	9	10
Line Total Check	Line Label	Bracing Element No.	Available Wall Length L (m)	Angle to Bracing line (degrees)	Element Height H (m)	Bracing Type	Supplier	Bracing Units Achieved	
								W	E
272	a	1	2.4		2.4	GS1-N	GIB®	166	144
		2	2.4		2.7	GS1-N	GIB®	147	128
267	b	1	1		2.4	BL1-H	GIB®	118	103
			0.9		2.7	BL1-H	GIB®	91	82
			0.9		2.7	BL1-H	GIB®	91	82
								Wind	Earthq.
Totals Achieved		W	260%	EQ	389%	612	539		
Timber Floor, design limit of 120 BU/m						<b>accepted</b>		OK	OK
Totals Required (from Demand)								235	138



**Fabricator / Designer Statement**

Job Ref:

7578C

This statement may be used by the Building Consent Authority for compliance purposes and is issued by a licensed truss fabricator using the Pryda Build software.

**CLIENT Name:** Mr & Mrs Morton-Jones

**SITE Details:**

Address: 52 Kauri Point Rd  
Laingholm

City:

Post Code:

**Nominal Design Criteria:**

Design working life: 50 years

Building importance: Residential

Roofing: Corrugated iron (8.0 kg/sq.m)

Ceiling: 10mm Gib-board (6.8 kg/sq.m)

Top chord purlins: 900 mm

BC restraints: Battens at 600 mm

Standard truss spacing: 900 mm

Standard roof pitch: 45.00 deg.

Ult. design wind speed: 44 m/s (wind classification = High)

Design roof snow load: 0 Pa  
(incl. probability factor)

Ground snow load: 0 Pa

Location: Region N0 - upper Nth Island

Altitude above sea level: 100 m

Max. eaves height: 3 m

Max. ridge height: 6 m

Int pressure coeff. up: 0.2

Overhang Condition: No fascia

The correctness of the Design Criteria used by the Pryda Build truss design software is the responsibility of the fabricator.

Note: A structural fascia beam is required at all hip and dutch hip corners to support the short creeper/rafter overhangs, as shown in AS4440-2004

Note: The standard trusses in this job have been designed for wind conditions assuming a fully hipped (or Dutch hip) roof.

All truss designs and their connections have been designed using Pryda design software. Additional items such as roof/ceiling plane bracing, special notes, supplementary timber, etc., which may be shown on the plan drawings are the responsibility of others.

All trusses shall be manufactured in accordance with the fabrication specifications provided by Pryda, and installed, connected and braced in accordance with the recommendations given in - : AS4440:2004 "Installation of nailed timber roof trusses" and any other supplementary details that may be provided, such as the Pryda Installation Guides.

Timber verification and grading values are in accordance with clause B1 and timber treatment in accordance with clause B2 of the New Zealand Building Code.

I/we confirm that the trusses for this project have been manufactured in accordance with the fabrication specifications provided by Pryda New Zealand

Name: BRIAN BURNSIDEPosition: TRUSS DETAILERSigned: B. BurnsideDate: 04-11-2014



**Producer Statement - PS1 - Design**

Job Ref: 7578C

This producer statement applies to the structural engineering design software "Pryda Build" supplied by Pryda NZ to

**HENDERSON TIMBER**

who is licensed to use the software to produce nailplated timber roof truss, floor truss, lintel and beam designs. These truss designs are in accordance with sound and widely accepted engineering principles and comply with the Compliance Document for the New Zealand Building Code, Clause B1, and New Zealand Building Code Verification Method B1/VM1. The durability shall comply with the New Zealand Building Code, Clause B2, for importance level 2 and a design working life of 50 years.

In addition to the above, this software also complies in part with:

ANSI / TPI 1 - 2002 National Design Standard for metal plate connected wood truss construction.

AS 1649 - 2001 Timber - Methods of test for mechanical fasteners and connectors - Basic working loads and characteristic strengths.

The truss designs require that the supporting structure is stable in its own right, and that the trusses will be braced in accordance with the New Zealand Building Code Standard NZS 3604:2011, and any supplementary details provided, such as the Pryda Installation Guides.

Pryda NZ holds a current policy of Professional Indemnity Insurance with cover no less than NZ\$2 million. The policy includes the engineering design processes used in the software.

On behalf of Pryda NZ (a division of ITW New Zealand)

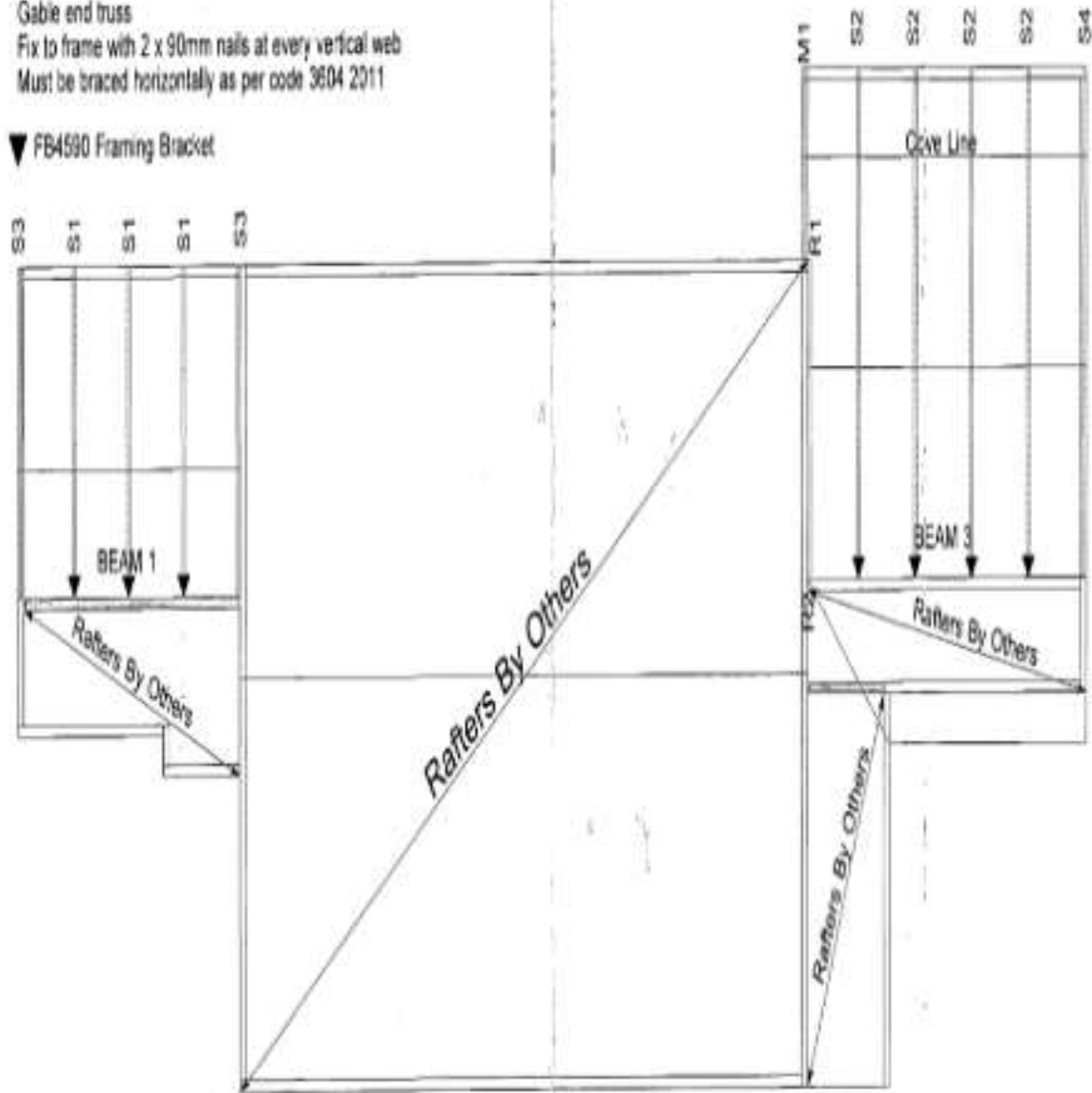


A C van Blerk BSc (Eng) MIPENZ (214689) CPEng IntPE  
Engineering Services Manager

All tie-downs Use 2 / 90 x 3.15 dia Skew Nails  
with 2 x Z nails unless otherwise noted

Gable end truss  
Fix to frame with 2 x 90mm nails at every vertical web  
Must be braced horizontally as per code 3604 2011

▼ FB4590 Framing Bracket



Preliminary Roof Layout for Council Consent Application only  
Not for manufacture